AGENDA ITEM 9

COMSTOCK HISTORIC DISTRICT COMMISSION P.O. BOX 128 VIRGINIA CITY, NEVADA 89440

APPLICATION FOR CERTIFICATE OF APPROPRIATENESS

Pursuant to Nevada Revised Statutes Section 384.110, application is hereby made to the Comstock Historic District Commission for a Certificate of Appropriateness for work I propose to undertake as described below:

propose to undermine de sier in a
Property address/description246 D Street Located in the community ofVirginia City
Description of proposed work: New StructureGarage
□Alteration of / Addition to Existing Structure
□Move Existing Structure
(Reason)
□Demolish Existing Structure
(Reason)
By making this application I hereby agree to indemnify and save and hold harmless the State of Nevada and the Counties of Lyon and Storey and their agents and employees from any and all claims, causes of action or liability arising from the granting of this application. I further agree to strictly comply with any and all conditions of the <i>Certificate of Appropriateness</i> , if issued, and the regulations and laws of the Comstock Historic District Commission.
Owner or Designated Representative: Name_TED_ELSCILL Mailing Address 1361 HORSE CKEEK WAY FERWEY NJ 89408 Signature
CHDC Staff: Received By Title Comstock Preservation & Date April 2024 History Officer



Proposed garage location behind house



View from D Street



View from C Street











From: Fernley Tire & Brake
To: Kristen Brown
Subject: Color of Garage

Date: Monday, April 22, 2024 8:18:20 PM

WARNING - This email originated from outside the State of Nevada. Exercise caution when opening attachments or clicking links, especially from unknown senders.

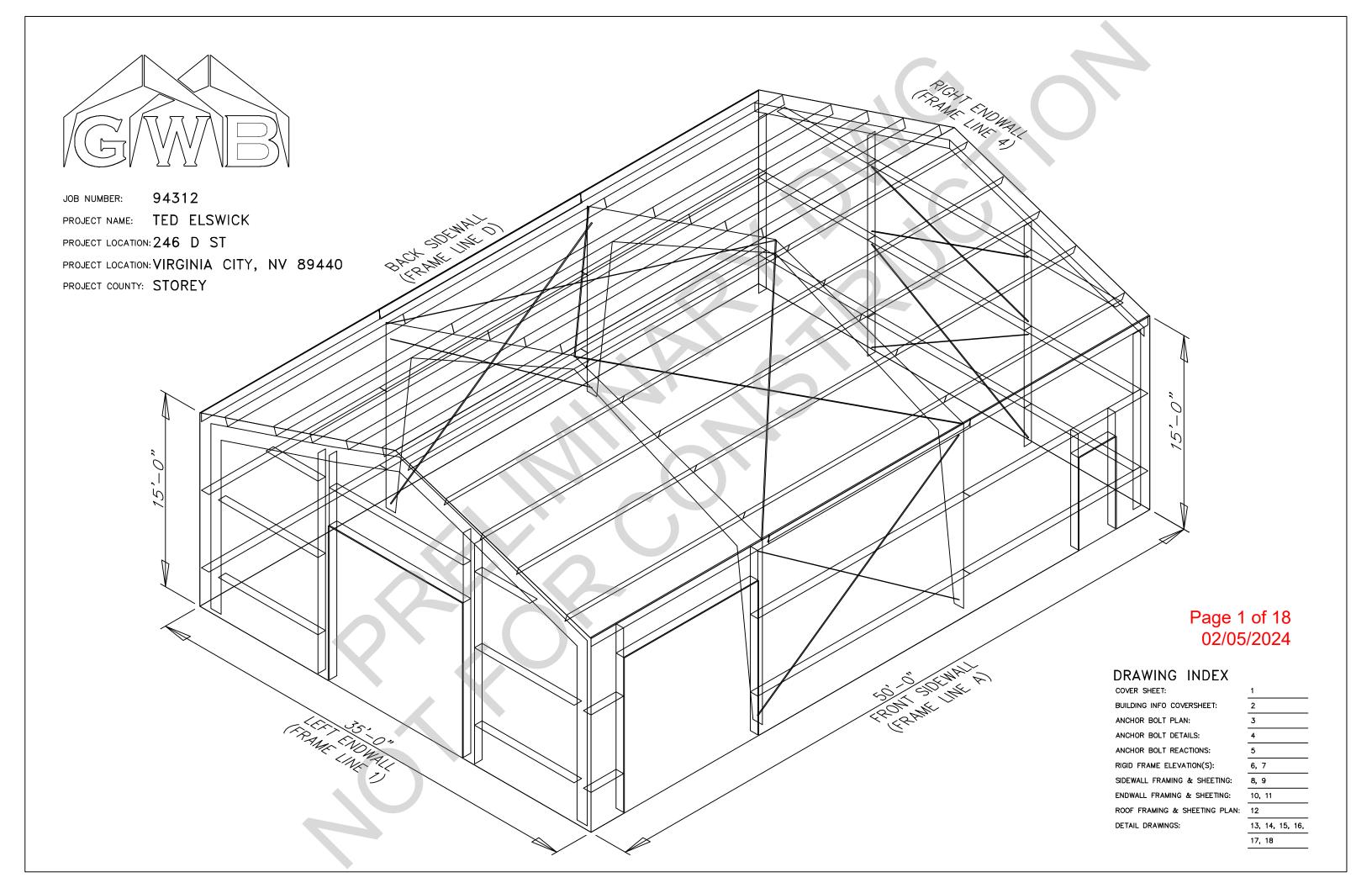
Hello

The new building for our property at 246 N. D Street Virginia City, NV will be the color of galvalume. This color galvalume will the color of roofing, sides, and will paint roll-up doors silver.

I appreciate your assistance and consideration

Best regards,

Ted Elswick



GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HÍGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH

Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & LESS) A307 A325-TYPE 1 HIGH STRENGTH BOLTS (>1"Ø TO 1 1/2"Ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT : A) ENDWALL RAFTER SPLICE - 5/8" × 0'-1 3/4" A325-N
B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE— BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO
PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES

STUCKLAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE MISTIS. THE OFFICE THE OF MINOR MISTIS BY THE USE OF MINITERIST HE OFFI THE OFFI OF MINITERIST. OF THE MINITERIST HE OFFI THE OFF

BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE DECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION

THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN (CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEAS FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTIC LATEST ENTITLE).

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. F. COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMI SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED THE ERECTION OPERATION SHALL BE DETERMINED, PURINSHED AND INSTALLED BY THE EINO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STAIL BRACTE LATERS EDITION). PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEA BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO TO

CONCRETE.

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESI
IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATIO
FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT
ADEQUATE PROMISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY
REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE S
AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHOR,
FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER
EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING
SYSTEMS MANUAL) SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODER AMOUNTS OF REAMING, CHIPPING, WELDING OR CULTING, AND THE DRAWING OF ELEMENT INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED B' FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHO IS RESPONSIBLE ETHER TO CORRECT THE ERROR OR TO APPROVE THE MOST FEICIENT ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST ENTION).

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OF ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WC SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITIO

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction lead or copper, both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel pan even run-off from copper flashing, wiring, or tubing onto galvalume should an expensive the panels of the property of the panels of the pa

2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENIPERACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFET HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFET HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFET BUILDING, EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEET HIGHLIGHTING SAFETY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEET HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HADN HATS, SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFET WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTR TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DE CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AN ICE BUILDUP. SSE TABLE BELOW. ICE BUILDUP, SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22,45	11,22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 200 EDITION. SECTION A8.4. PAGE XI-A8-2

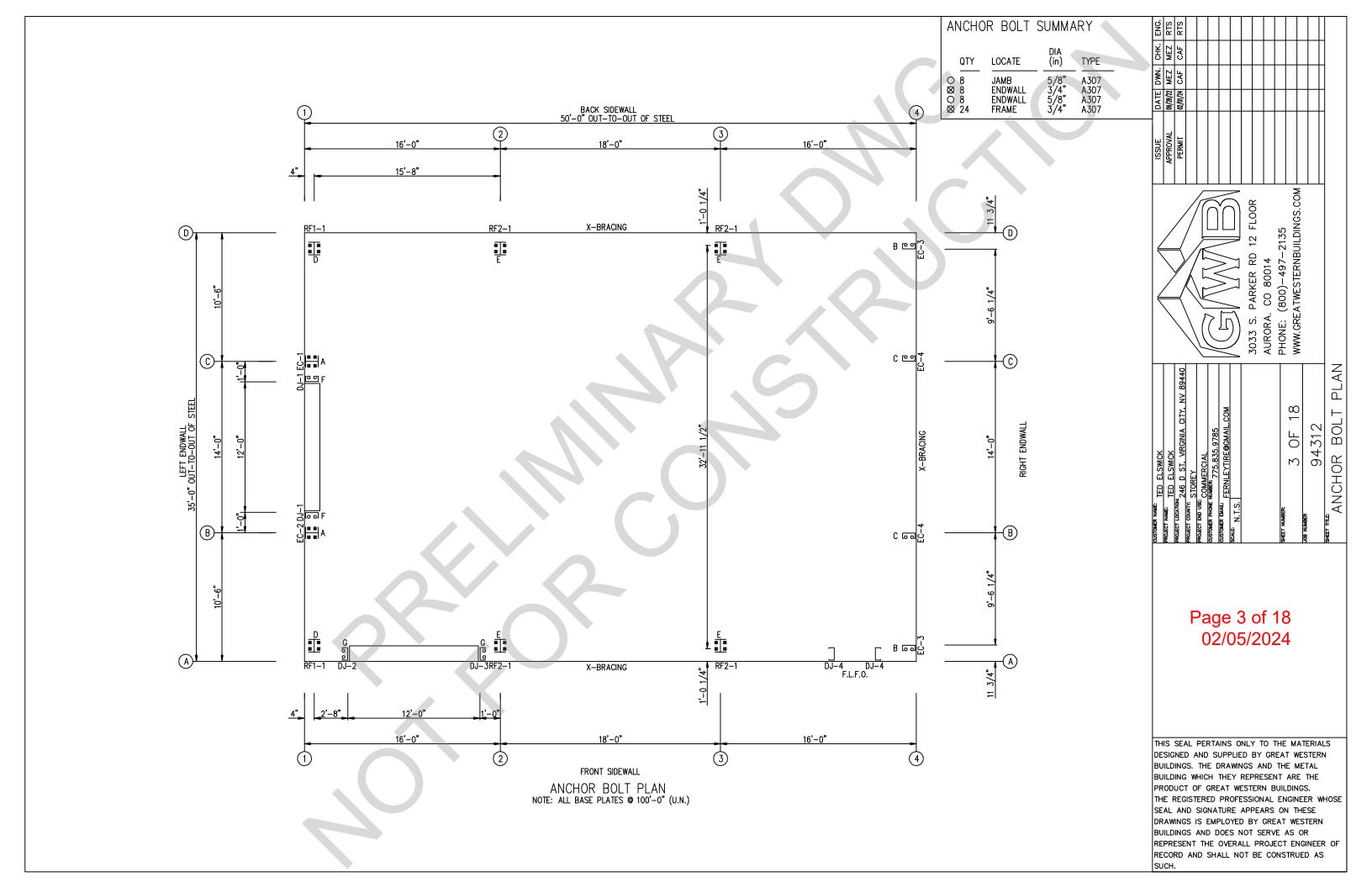
WALL INSULATION

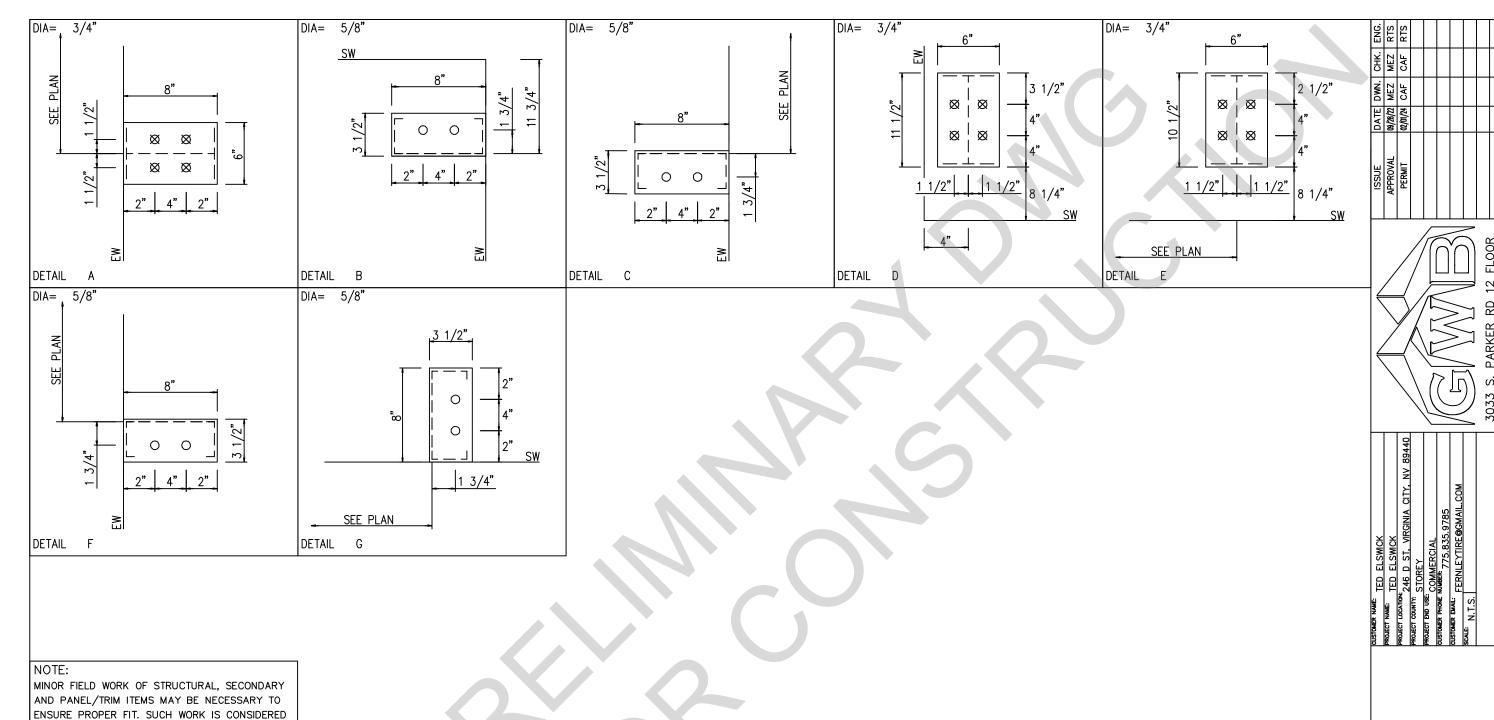
:N/A

		BUILDING LOADS	R R R IS G
ION.	THIS STRUCTURE HAS BEEN DESIGNED	O IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:	CHK.
ARE TO BE N CRITERIA EASE FOR CTICE, S. PLANS ITIES. DING N R A EN OF THE	DESIGN LOADS: DESIGN CODE / WIND CODE OCCUPANCY / RISK CATEGORY ENCLOSURE ROOF DEAD LOAD (D) (PSF) ROOF COLLATERAL LOAD (C) (PSF) WIND LOAD	: IBC 18 / IBC 18 : II — Normal : Enclosed : 10.0 : 1.00	DATE DWN. 29/28/22 MEZ 22/01/24 CAF
ND IEMPORARY QUIRED FOR E ERECTOR. IG ANCHOR STANDARD TO PERMIT IEAR, E BOLT THE	ULTIMATE WIND SPEED, (VULT) (MPH) WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF) WIND ENCLOSURE CLASSIFICATION LIVE LOAD PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF)	: 115.0 : C : 0.18/-0.18 : 23.90/-25.93 : Enclosed : 20.00 : No : 20.00	12 FLOOR UILDINGS.COM
DESIGN AND ATION OR HAT BY COLUMN IE SOIL MIDDRAGE AND EER ING DEER ATE ENTS D BY THE ARE TO BE MHOEVER	SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct) SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION	: 70.00 : 70.0 : 1.00 : 1.00 : 1.00 : d : Ss = 1.586 :S1 = 0.553	S. PARKER RD RA. CO 80014 E: (800)-497- SREATWESTERNB
OTHERWISE ES, UNLESS WORK IS ETE OR TO HON) ON WITH THE ANELS.	SPECIFIAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM TOTAL DESIGN BASE SHEAR, (V) (KIPS)	: Sds = 1.366 :S1 = 0.333 : Sds = 1.269 :Sd1 = 0.644 : D : STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE : RIGID FRAMES (OMF) : BRACED FRAMES (OCBF/OMF) : LONGITUDINAL = 19.53 : TRANSVERSE = 19.67	COM COVER
TURE ETY OF M.B.S. VENTION AFETY AND ITY, MAKE NG A AEETINGS ITS, RUBBER AFETY NETS STRUCTION	RESPONSE MODIFICATION FACTORS, (R) SEISMIC RESPONSE COEFFICIENTS, (Cs) ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	: RIGID FRAMES = 3.25 Ω = 3.00 Ω = 2.00 : SW X-BRACING = 3.25 Ω = 2.00 : RIGID FRAMES = 0.3906 : SW X-BRACING = 0.3906 : EQUIVALENT LATERAL FORCE PROCEDURE	WE TED ELSWICK FIND TED ELSWICK MINNE 246 D ST. VIRGINIIA USE COMMERCIAL ONE NUMBER 775,835,9785 MILE FERNLEYTIRE ©CMAIL S. SUILDING IN
RED OF DEPTH AND/OR CIGHT D START	BUILDING DESCRIPTION: WIDTH (FT) : 35.0 LENGTH (FT) : 50.0 EAVE HEIGHT AT BSW (FT): 15.0 EAVE HEIGHT AT FSW (FT): 15.0 ROOF SLOPE AT BSW : 4.0:12 ROOF SLOPE AT FSW : 4.0:12 BAY SPACING (FT) : 1 AT 16, 1 AT 18, 1	AT 16	Page 2 of 18 Consider in the page 2 of 18 Consider 19 on the page 3 on the page 3 on the page 4
2002	COVERING AND TRIMS: ROOF PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : GALVALUME TRIM COLORS GABLE/EAVE : ASH GRAY EAVE GUTTER : ASH GRAY WALL PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : HAWAIIAN BLUE TRIM COLORS : ASH GRAY CORNER : ASH GRAY FRAMED OPENING : ASH GRAY DOWNSPOUTS : ASH GRAY HAWAIIAN BLUE INSULATION ROOF INSULATION : N/A		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN

BUILDINGS AND DOES NOT SERVE AS OR

REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS





Page 4 of 18 02/05/2024

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THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDINGS WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

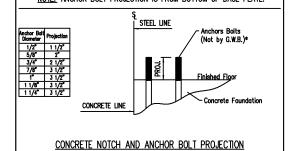
G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

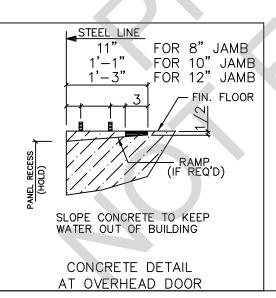
ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

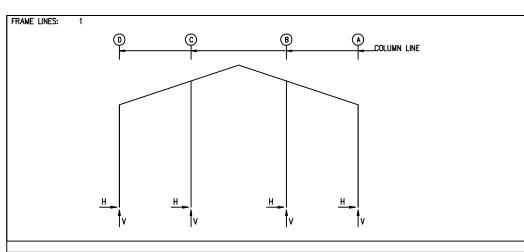
A NORMAL PART OF METAL BUILDING ERECTION.

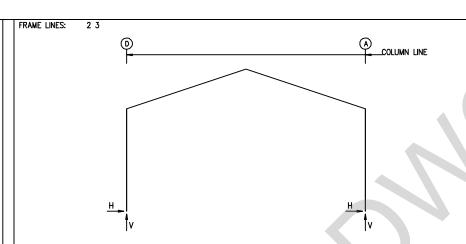
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.









RIGID	FRAN	ΛE:	BASI	C COLUM	IN REACT	IONS (k)								
FRAME Line 1 1 1 1	A C	Horz 0.0	Dead Vert 0.7 0.7 1.5 1.5	Horz 0.0	Vert 0.0	Horz 0.0 0.0 0.0 0.0	Vert 0.7	Horz 0.1 -0.1 0.0 0.0	-Snow Vert 2.5 2.5 7.5 7.5	Horz -1.1	_Left1- Vert -2.0 0.3 -0.2 -2.5	-Wind_ Horz 1.3 1.1 0.0 0.0	Right1- Vert 0.3 -2.0 -2.5 -0.2	
FRAME Line 1 1 1	Column Line D A C B	Horz -1.4	Left2- Vert -1.3 1.0 0.1 -2.2	Horz 0.9 1.4 0.0	Right2- Vert 1.0 -1.3 -2.2 0.1	HorzOP 0.0 0.0 -2.0	0.0	HorzOP 0.0	0.0	Horz 0.7	Vert -1.0 -1.4 -1.5	Wind Horz 0.4 -0.7 0.0 0.0	I_Long2- Vert -1.4 -1.0 -0.2 -1.5	
FRAME Line 1 1 1	Column Line D A C B	11	ic_Left Vert -4.1 4.1 5.1 -5.1		Right Vert 4.1 -4.1 -5.1	-Seism Horz 0.0 0.0 0.1 0.1	Vort	Horz 01	SL_L- Vert 2.4 0.5 8.6 3.4	Horz 0.1	SL_R- Vert 0.5 2.4 3.4 8.6			
FRAME Line 2* 2*	Column Line D A	Horz 1.2	Dead Vert 3.7 3.7	-—-Collo Horz 0,1	iteral— Vert	Horz 2.1 -2.1	Vert	Horz 7.3 -7.3	-Snow Vert 20.8 20.8	Horz	Vert -5.9	-Wind_ Horz 1.4 3.5	Vert -3.2	
FRAME Line 2* 2*	Column Line D A	Horz	_Left2- Vert -3.7 -1.0	Horz 1.0	Right2- Vert -1.0 -3.7	Horz	_Long1- Vert -6.3 -6.0	Wind Horz 0.1 -0.7	I_Long2- Vert -6.0 -6.3	Horz -4.2	Vert −3.5	Horz	_Right Vert 3.5 -3.5	
FRAME Line 2* 2*	Column Line D A	Horz	Vert	Horz	SL_L- Vert 19.8 11.4	F2UNB_ Horz 5.8 -5.8	SL_R- Vert 11.4 19.8							
2*	FRAME lin	ies:	2 3											

RIGID	FRAME:		MAXIMUM	REACTION	S, ANC	HOR BOLT	S, & BASI	E PLATI	ES					
Frm Line	Col Line	Load Id		umn_React V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	:_Plate(in) Length	Thick	Grout (in)	
1	D	3	1.6	3.7	4	-1.5	-2.6	4	0.750	6.000	11.50	0.375	0.0	
1	A	5 2	1.5 -1.6	-2.6 3.7	2 5	-1.6 1.5	3.7 -2.6	4	0.750	6.000	11.50	0.375	0.0	
1	С	7 9	1.3	-0.6 10.2	8 5	-1.2 0.0	-0.4 -3.0	4	0.750	6.000	8.000	0.375	0.0	
1	В	10 9	1.3 0.0	-0.6 10.2	11 4	-1.2 0.0	-0.4 -3.0	4	0.750	6.000	8.000	0.375	0.0	

RIGID	FRAME:		MAXIMUM	REACTION	S, ANCI	HOR BOLT	S, & BASE	PLATE	ES				
Frm Line	Col Line	Load Id	Hmax H	imn_React V Vmax	ions(k Load Id	Hmin H	V Vmin	Bolt	t(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)
2*	D	1	8.6	24.8	4 6	-2.4 0.5	-0.9 -5.0	4	0.750	6.000	10.50	0.375	0.0
2*	A	5 1	2.4 -8.6	-0.9 24.8	1 6	-8.6 -0.5	24.8 -5.0	4	0.750	6.000	10.50	0.375	0.0
2*	FRAME lin	es:	2 3	>									

		TRAME	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,													
4	ENDWA	ALL CC	LUMN:		BASIC C	OLUMN R	EACTION	S (k)								
	Line L	ine Ve A 0 3 1 3 1	ert ' 4 3 3	Collat Vert 0.0 0.1 0.1 0.0	Live Vert 0.6 2.2 2.2 0.6	Snow Vert 2.2 7.8 7.8 2.2	_	Wind_Left1 Horz Vo 0.0 -0 1.4 -4 0.0 0 0.0 -0	ert 1.6 1.2 1.2	0.0 1.4	Vert −0.9	Wind_Left2 Horz Vo 0.0 -0 -1.4 -3 0.0 0. 0.0 -0	ert).1 j.7 .8	Wind_Right2 Horz Vert 0.0 -0.4 0.0 0.8 1.4 -3.7 0.0 -0.1	Wind Press Horz -0.7 -1.9 -0.7	
	Line L 4 / 4 E 4 (Col Su Line Ho A 0.1	orz (8 1	0.0 0.5	Vert -1.2 -1.4 -1.7).0 - 0.5 -).0 -	g2 Vert -0.8 -1.7 -1.4 -1.2	Seis_Le Horz 0.0 -4.4 0.0 0.0	ft Vert 0.4 -5.7 5.4 0.0	Seis_F Horz 0.0 0.0 4.4 0.0	Right Vert 0.0 5.4 -5.7 0.4	Seis Long Horz 0.0 0.1 0.1	E2UN Horz 0.0 0.0 0.0 0.0	B_SL_L- Vert 2.2 9.0 3.4 0.4		
	Line L 4 / 4 E 4 (Line Ho. 3. 0. 3. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	0 3.4 0 9.0 0 2.2	t - -												
	ENDWA	ALL CC	LUMN:		MAXIMUM	REACTIO	NS, ANC	HOR BOLTS	6, & BA	SE PLATE	ES					
	Frm Line	Col Line	Load Id	Hmax H	lumn_Reac V Vmax	tions(k) Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Bas Width	se_Plate(in) Length	Thick	Grout (in)		
	4	A	14 1	0.5 0.0	-0.5 2.6	8 14	-0.4 0.5	-0.5 -0.5	2	0.625	3.500	8.000	0.250	0.0		
	4	В	7 15	1.3 0.0	-1.7 10.5	11	-1.2 0.0	-0.2 -3.5	2	0.625	3.500	8.000	0.250	0.0		
	4	С	10 16	1.3	-1.7 10.5	8 5	-1.2 0.0	-0.2 -3.5	2	0.625	3.500	8.000	0.250	0.0		
	4	D	17 1	0.5 0.0	-0.5 2.6	11 17	-0.4 0.5	-0.5 -0.5	2	0.625	3.500	8.000	0.250	0.0		

NOTES FOR REACTIONS

Building reactions are based on the following building data:
Width (ft)
Length (ft)
Eave Height (ft)
Roof Slope (rise/12)
Dead Load (psf)
Collateral Load (psf)
Live Load (psf)
Snow Load (psf)
Wind Speed (mph)
Wind Code
Exposure
Closed/Open
Importance Wind
Importance Seismic
Seismic Zone
Seismic Coeff (Fa*Ss) = 35.0 = 50.0 = 15.0/15.0 = 4.0:12/4.0:12 = 10.0 = 1.00 = 70.00 = 115.00 = IBC-18 = C = Enclosed = 1.00 = 1.00 = 0 = 1.90 ID Description

AN	ICHO	R BOLT S	SUMMA	.RY
0 8	QTY 8 8 8 8 24	JAMB ENDWALL ENDWALL FRAME	DIA (in) 5/8" 3/4" 5/8" 3/4"	A307 A307 A307 A307 A307

Reactions for seismic represent shear force, Eh

——Wа		- Col	<u>+</u>	React	ions(k)	smic -	Panel_	_Shear /ft)	
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Note
L_EW F_SW R_EW B_SW	1 A 4 D	2,3 B,C 3,2	2.5 1.4 2.5	1.9 1.7 1.9	12.7 4.4 12.7	9.4 5.3 9.4			(h)

Page 5 of 18 02/05/2024

3033 S. PARKER RD 12 FLOOR
AURORA. CO 80014
PHONE: (800)-497-2135
WWW.GREATWESTERNBUILDINGS.COM

REACTIONS

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ANCHOR

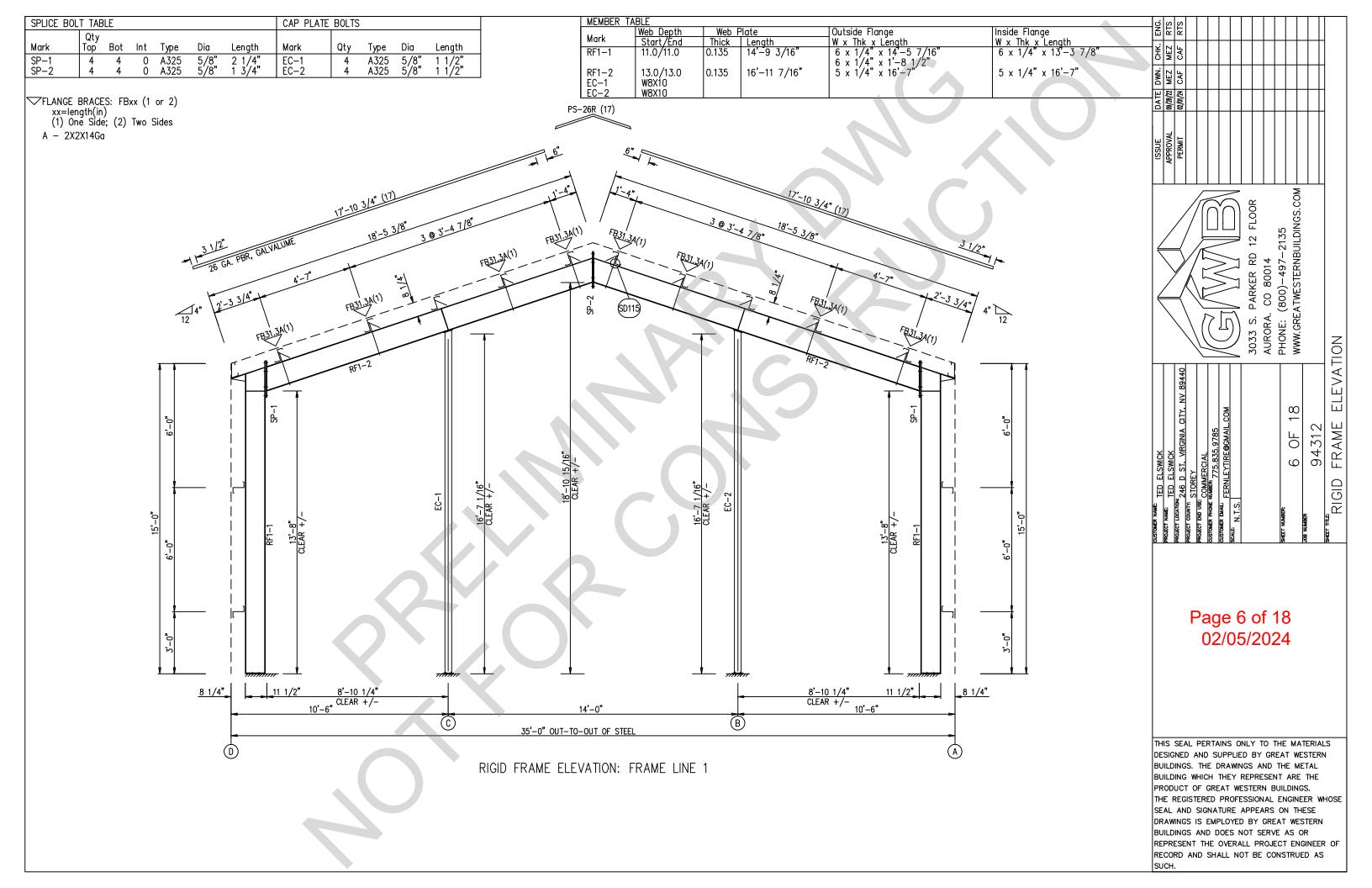
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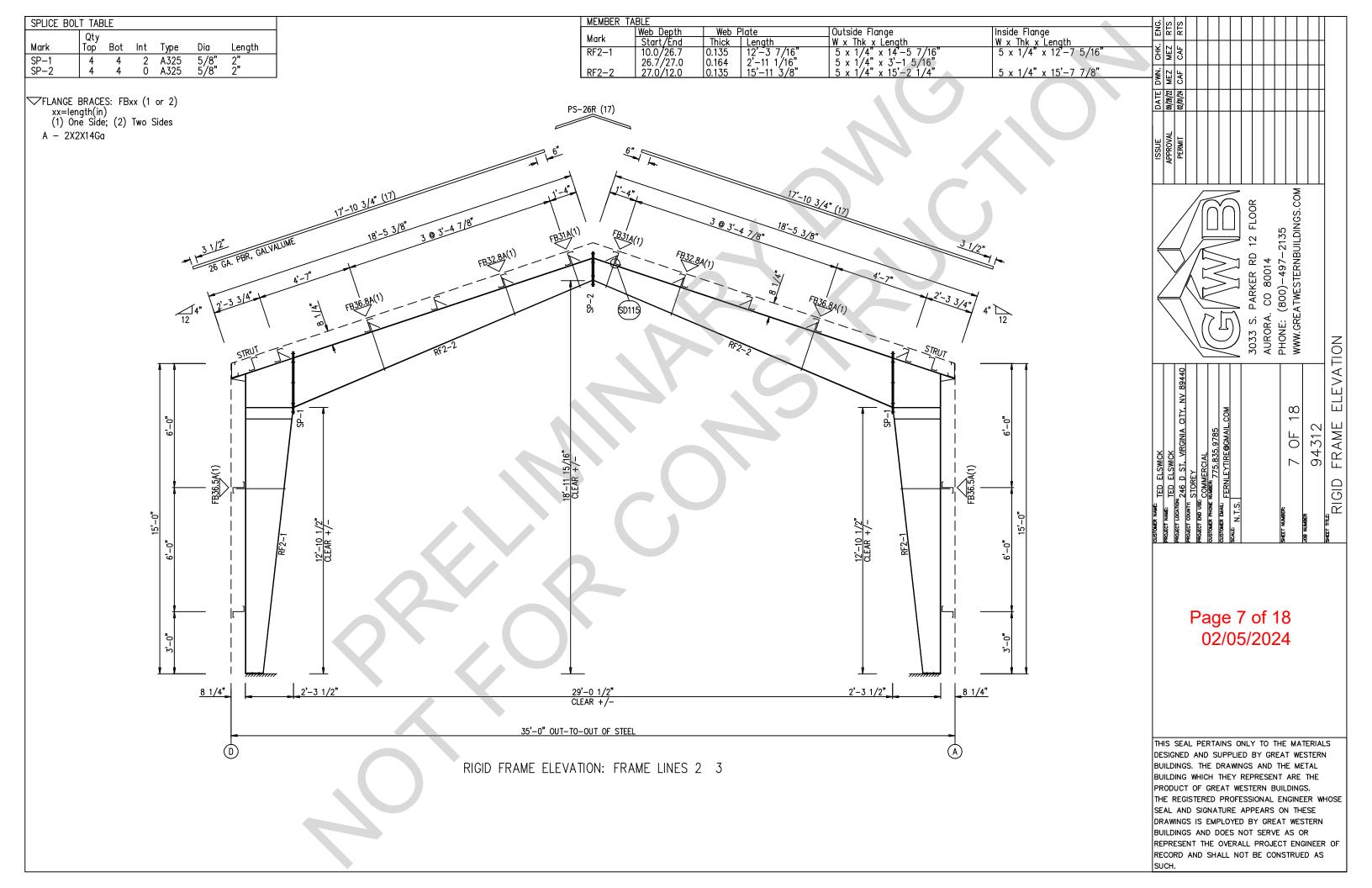
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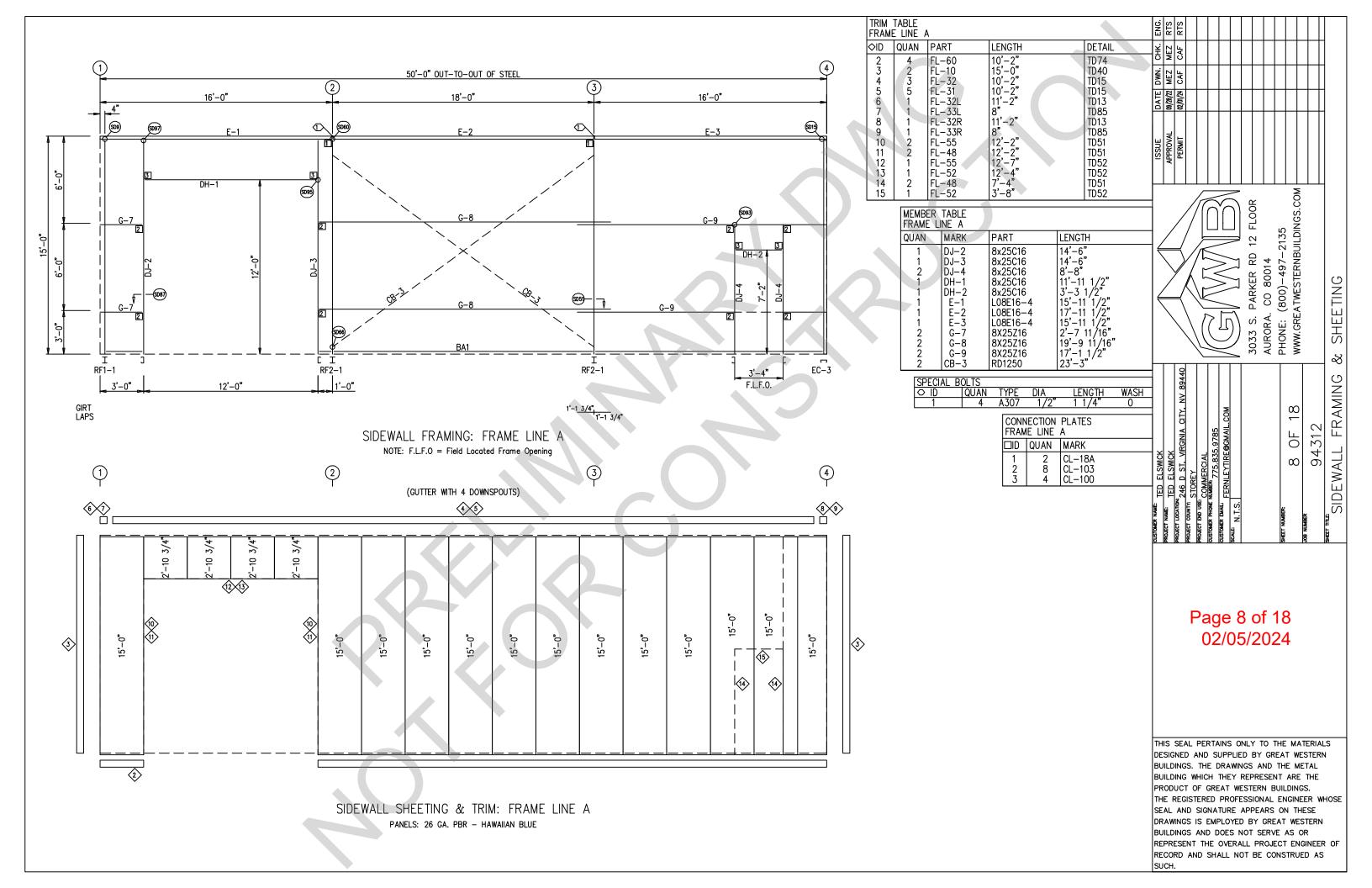
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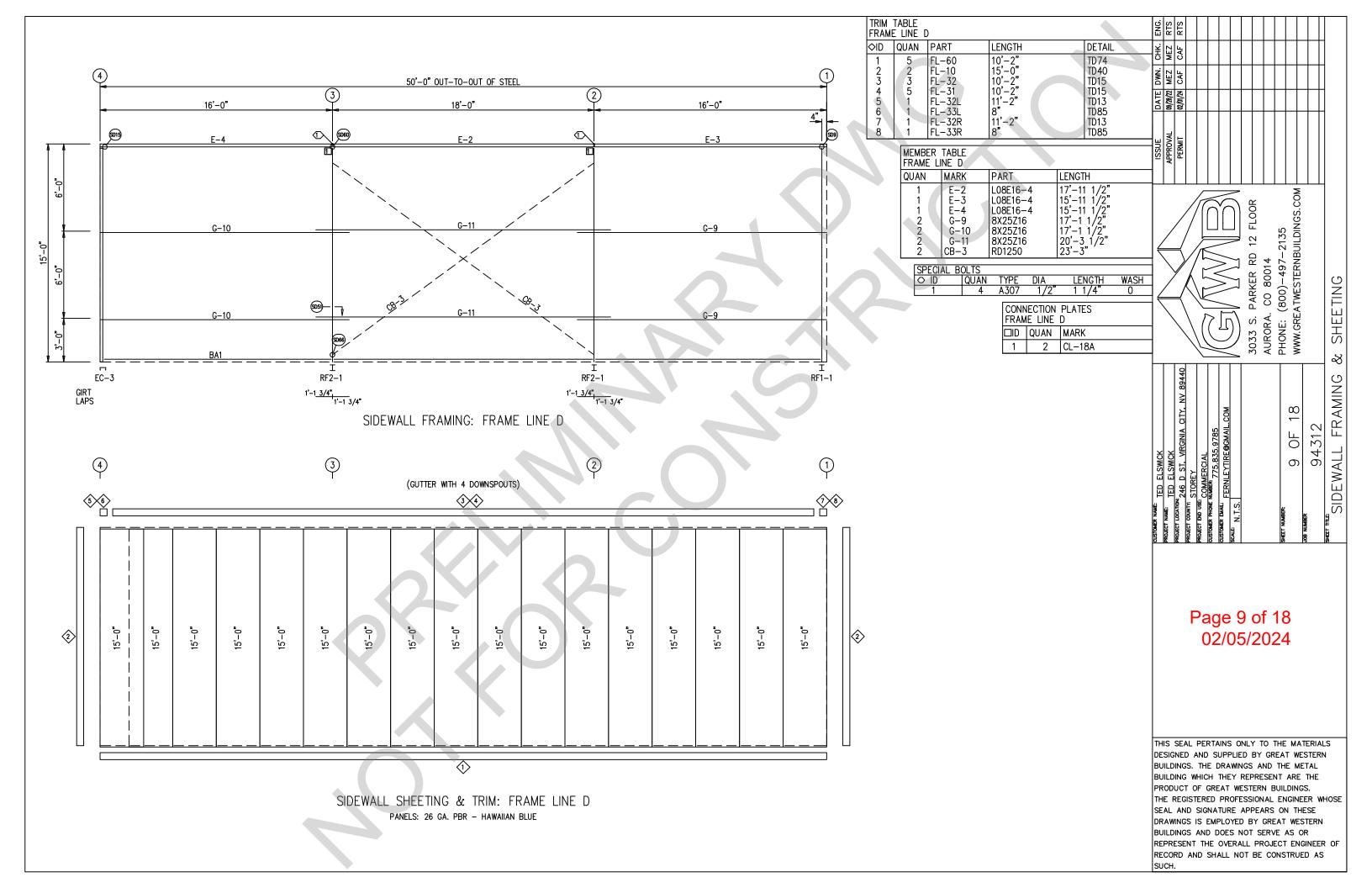
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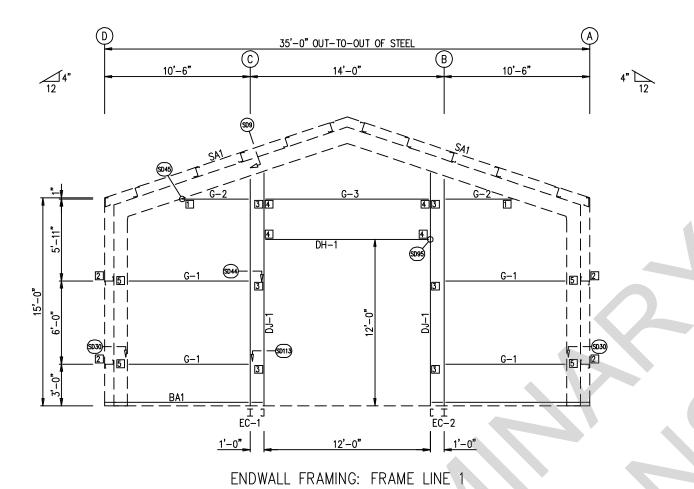
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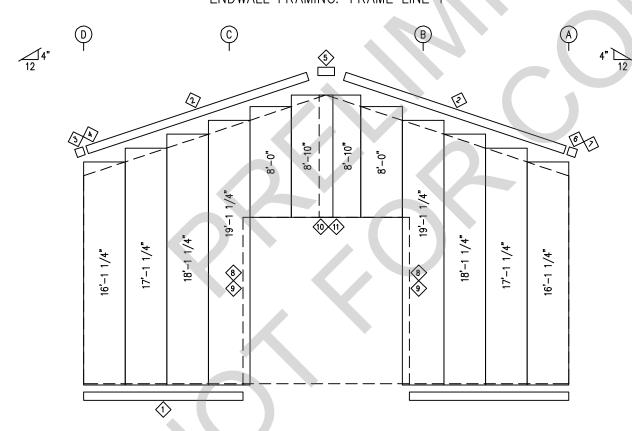






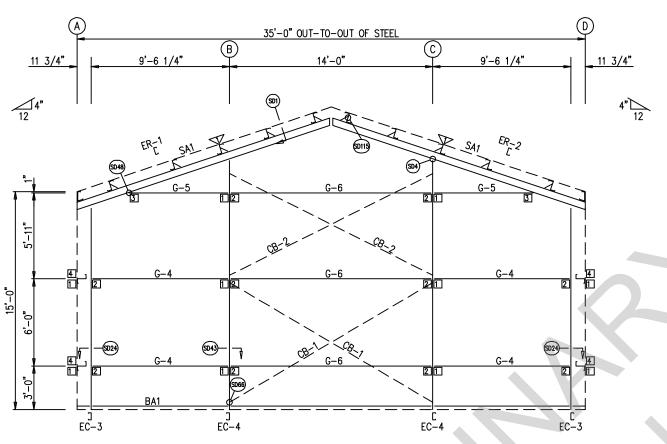




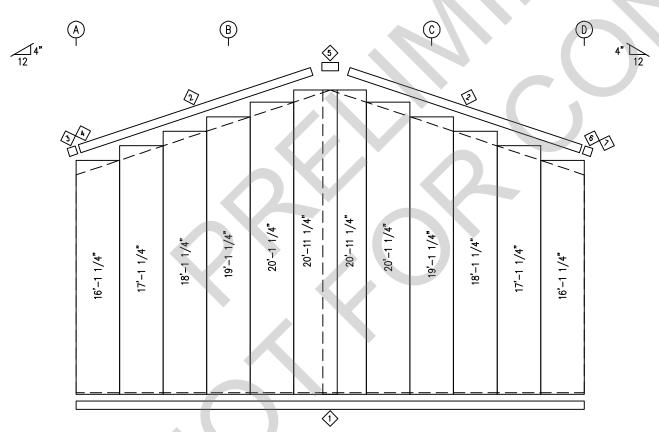


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - HAWAIIAN BLUE

TA	BLE 1						ENG.	RTS	STS			Τ		П			
	INE 1 UAN	PART	LENGTH		DETAIL		SK.	\vdash	CAF	H		+		H	++	+	
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	1	FL-21 FL-21L FL-328L	11'-2" 9, 1/2"		TD85 TD13		DATE DWN.	09/28/22 MI		+				H	+	+	
	1	FL-23 FL-21R FL-328R	11'-2"		TD85 TD13		ă	7/60	0/20						+	++	
	2 2 1	FL-55 FL-48	12'-2"		TD51 TD51		l u	۱۸۷	Ę								
	1	FL-55 FL-52	12'-7" 12'-4"		TD52 TD52		ISSNE	APPROVAL	PERMIT								
BOI FR	T TAE	BLE NF 1					\vdash			\coprod					 		
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JAN	IBS/R	/RAFTER AFTER	4 2	A325 5/ A325 5/	(8" 1 (8" 1	1/2" 1/2"			//]] غر	_ <u> </u>		12 FL		PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM		
	MEMBE FRAME	R TABLE LINE 1]€	_	$\frac{1}{2}$	/[\leq	_ :~		4	(800)-497-2135 ATWESTERNBUILDIN		
	QUAN 1	MARK EC-1	PART W8X10	LENG	TH 7 3/4"			\rangle	//	\ [\leq	\leq	PARKER RD	CO 80014)–49 :STER		ပ
	1 2	EC-2 DJ-1	W8X10 8x25C16	16'-1 16'-	7 3/4" 10 5/16"		$\mid \in$		\prec $^{\cdot}$		\leq			00	(800 ATWE		SHEETIN
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			CONNE FRAME	CTION PLAT	ES				9440								8
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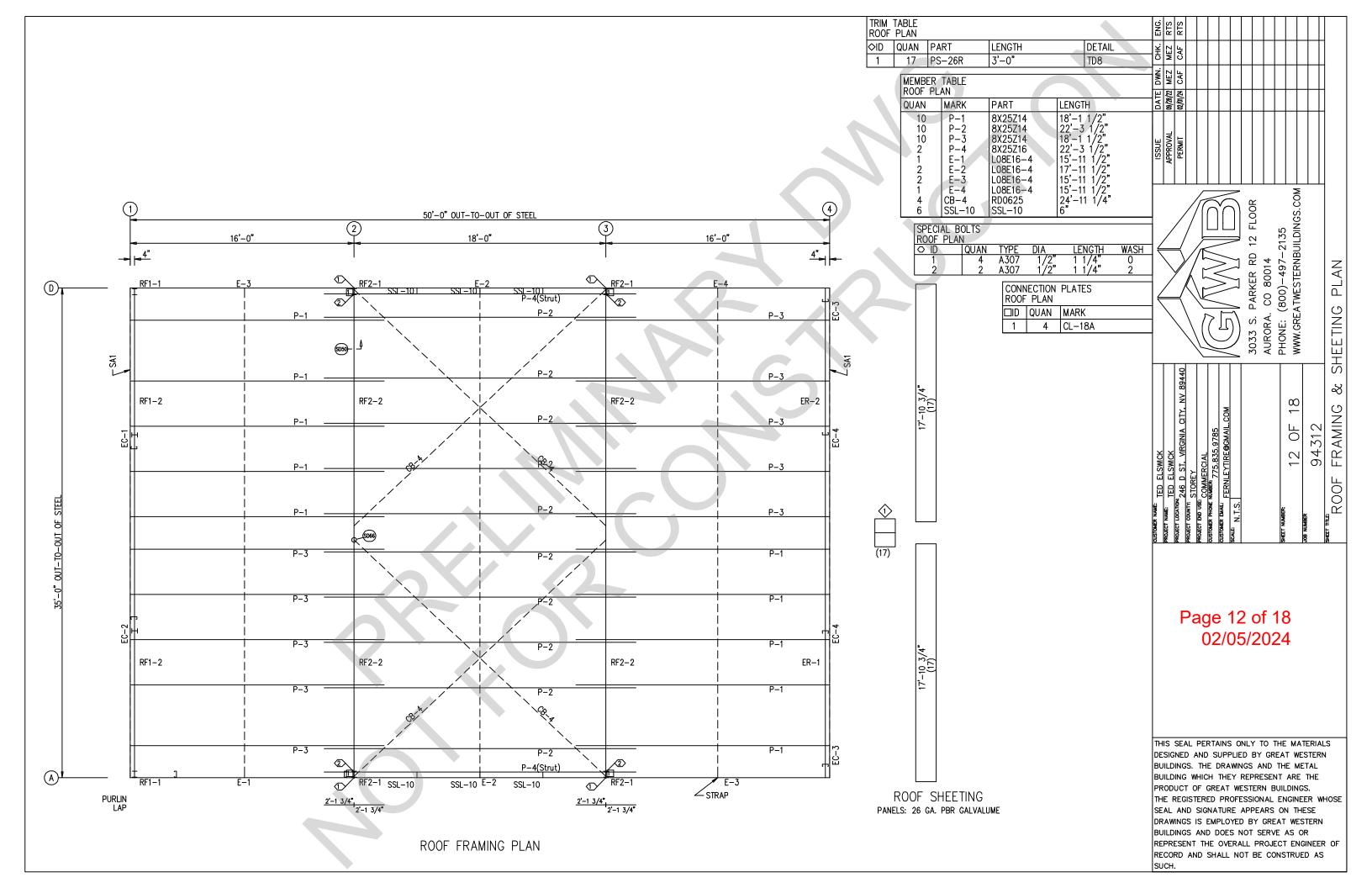
ENDWALL FRAMING: FRAME LINE 4

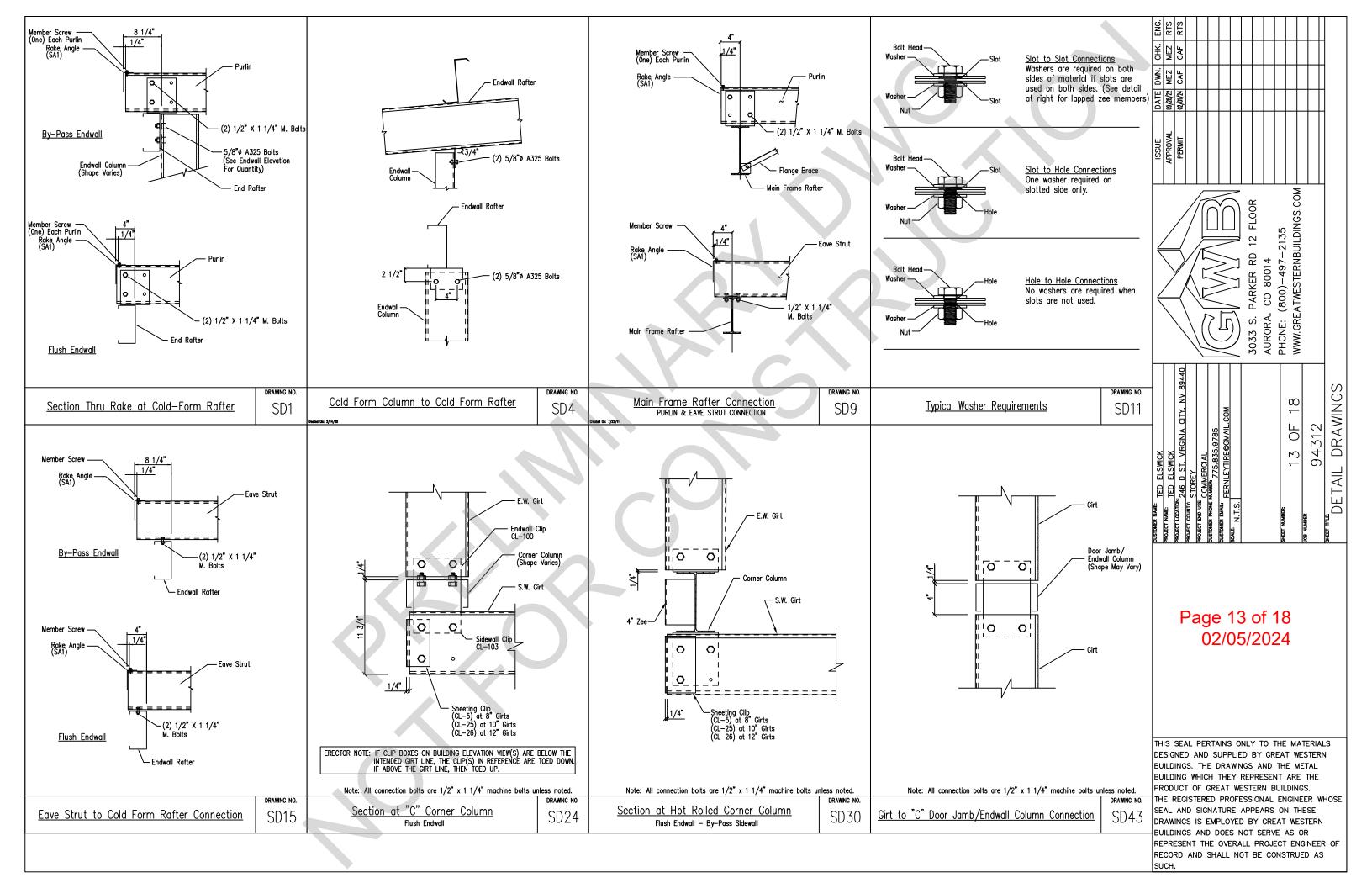


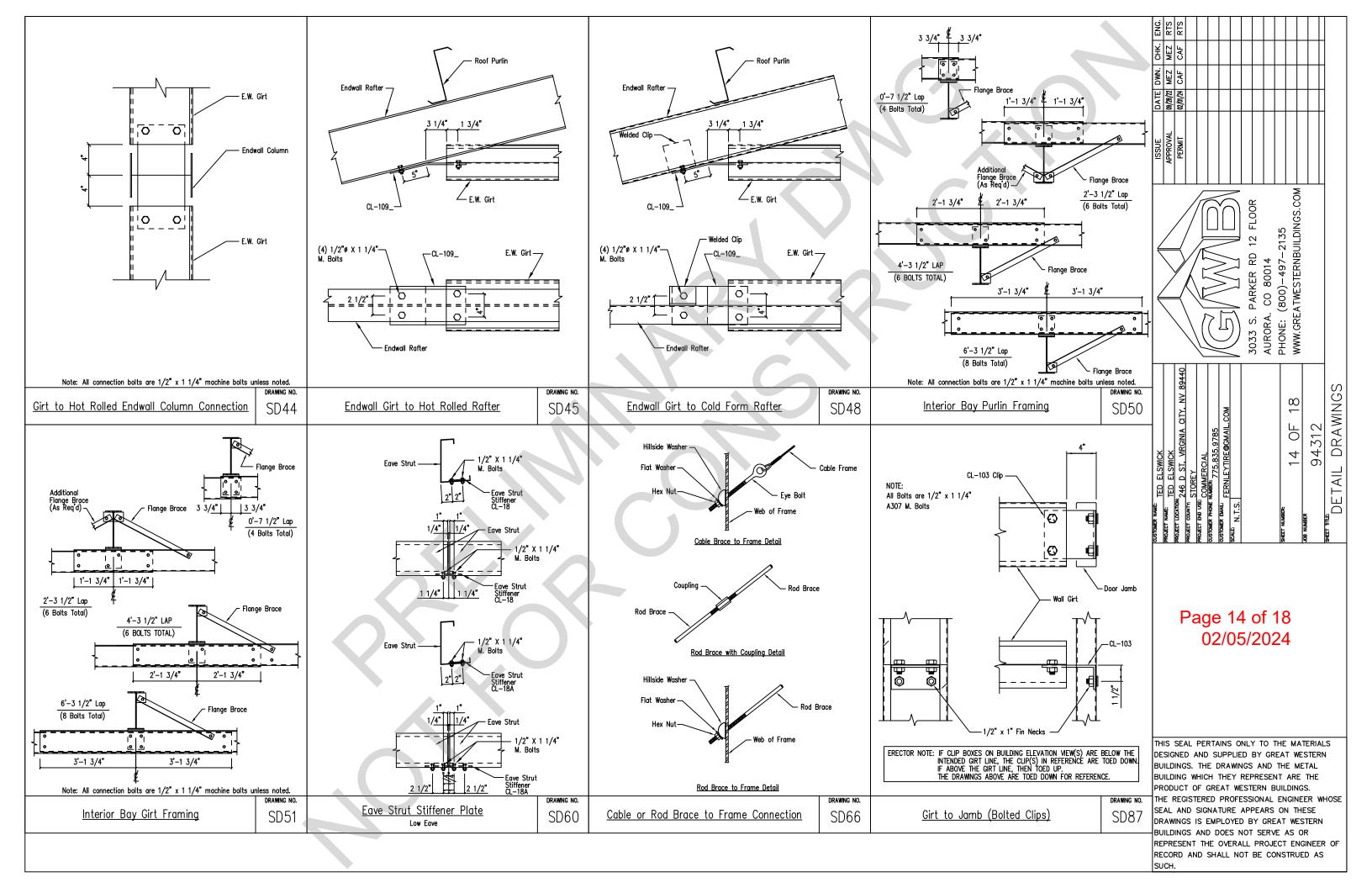
ENDWALL SHEETING & TRIM: FRAME LINE 4

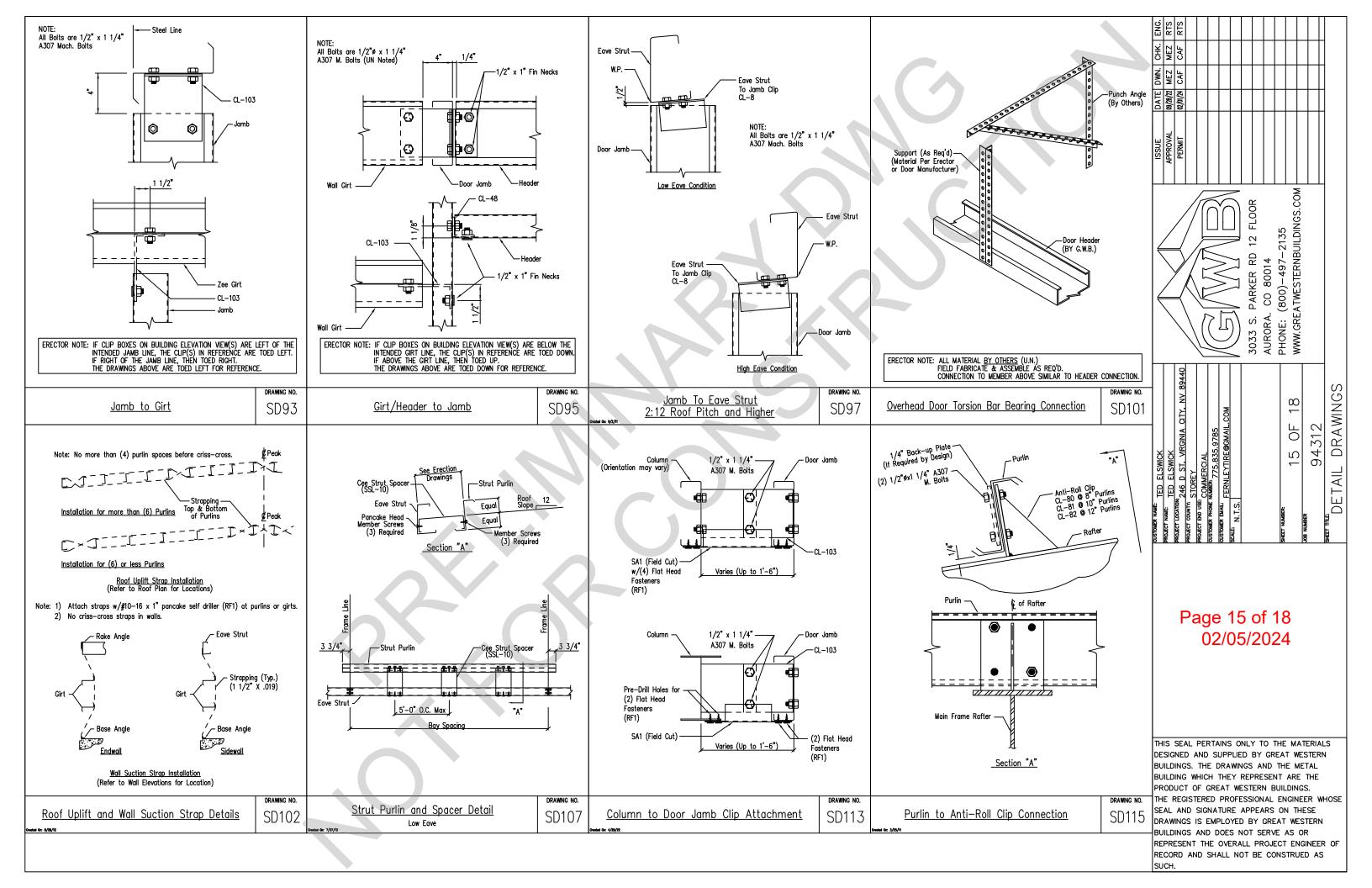
PANELS: 26 GA. PBR - HAWAIIAN BLUE

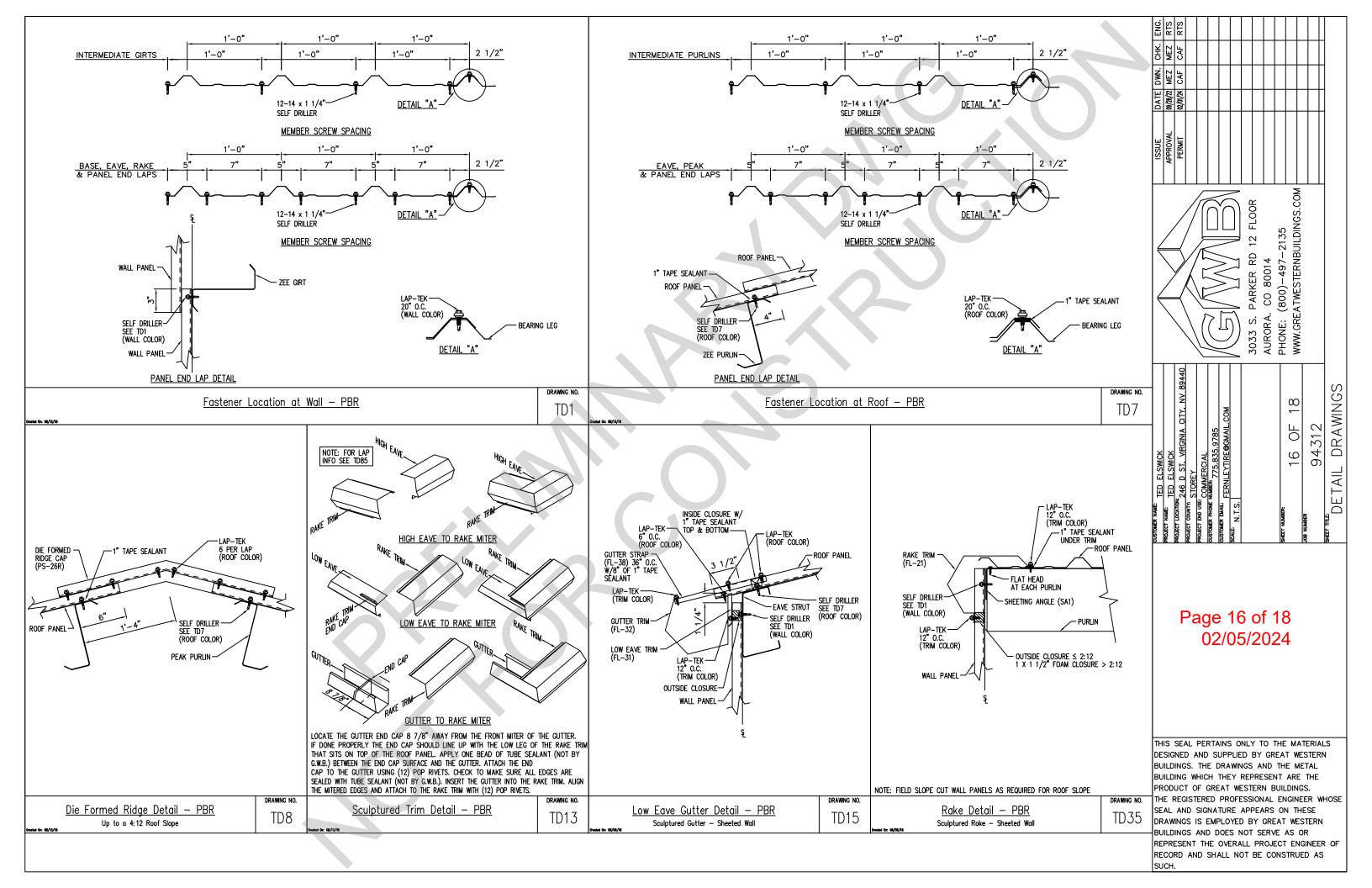
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1 2 3 4 5 6 7	3 FL-60 2 FL-21 1 FL-21L	10'-2" 8'-6" 11'-2" 9 1/2" 1'-4" 11'-2" 9 1/2"	TD35 TD85	DWN.	MEZ								
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L	MEMBER TABLE	1 2 1 1020	1 5/6 1 1/2			//		6	FLOOR		PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM		
	FRAME LINE 4 QUAN MARK	PART L	_ENGTH					Ц			35 DING		
	2 EC-3 2 EC-4	8x25C16 1	13'-7 15/16"] {			\subseteq	$\overline{}$	RD 12	_	(800)-497-2135 ATWESTERNBUILDIN		
	1 ER-1 1 ER-2	10x25C12 1 10x25C12 1	18'–5 1/8" 18'–5 1/8"		\searrow				유	CO 80014	-497 TERN		
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	2 G-5 G-6 2 CB-1 2 CB-2	8X25Z16 1 RD0625 1	16 - 10 18' - 5 1/8" 18' - 5 1/8" 9' - 1 15/16" 5' - 3 15/16" 13' - 11 1/2" 16' - 7 1/4"	`	\setminus	\	<u></u>	$\widehat{\mathbb{Z}}$	S	₹.	E: (8		SHEETING
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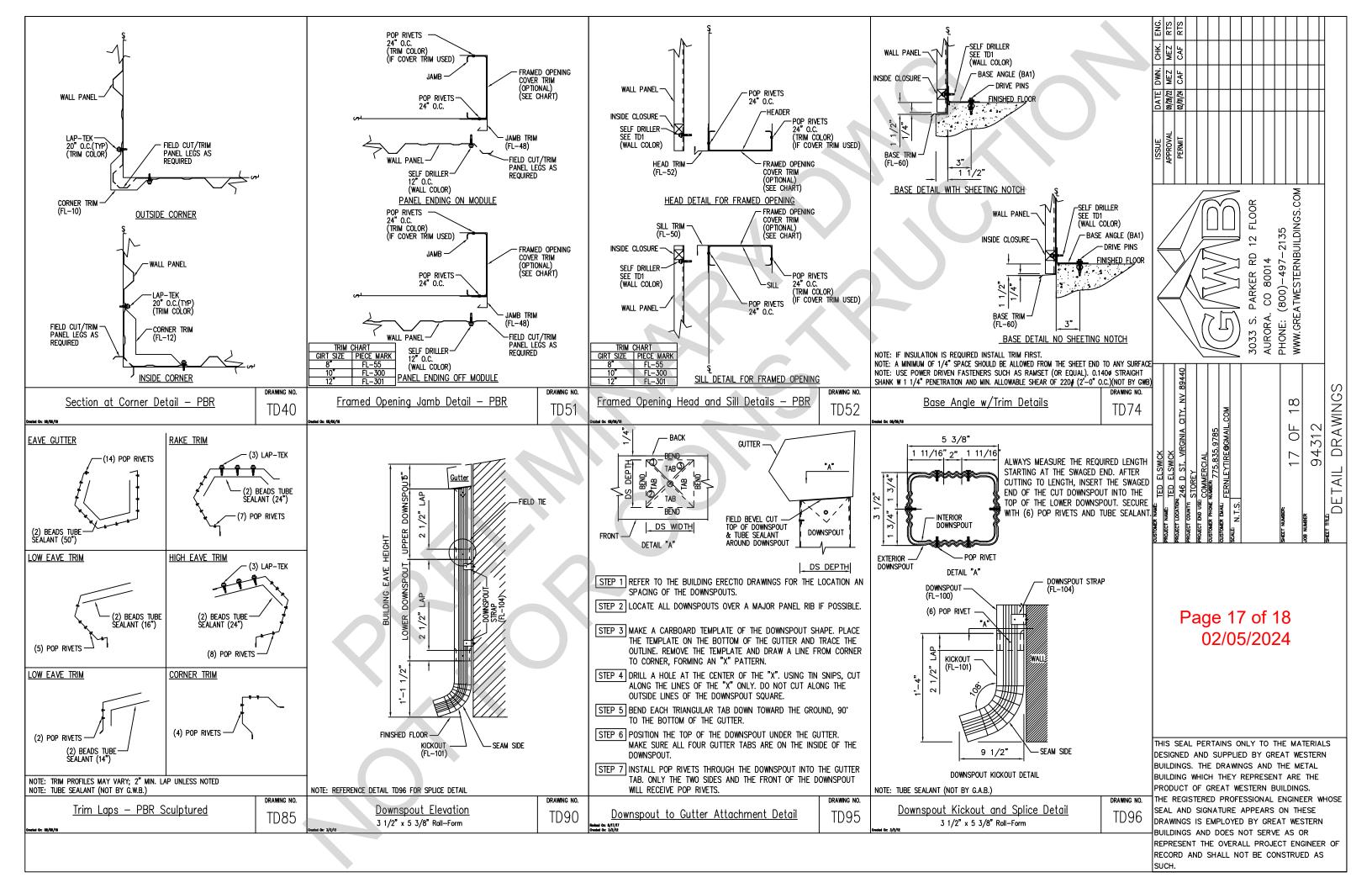


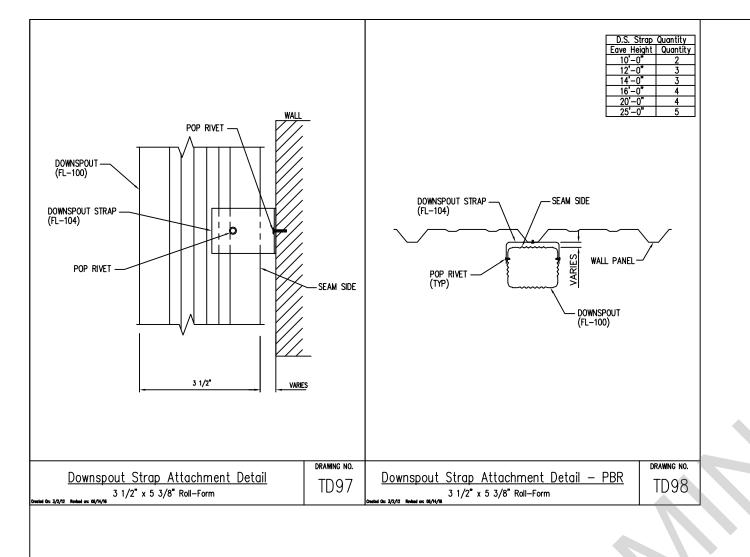


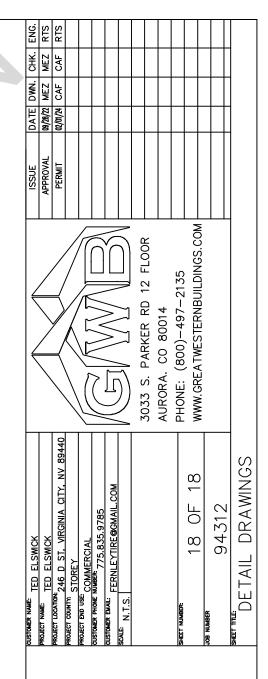












Page 18 of 18 02/05/2024

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