# AGENDA ITEM 8 246 N. D St. New Metal Garage Additional items requsted by the CHDC in May 2024

#### Kristen Brown

From: Fernley Tire & Brake <fernleytire@gmail.com>

Sent: Wednesday, May 22, 2024 8:48 PM

**To:** Kristen Brown **Subject:** 246 D st

**Attachments:** 246 N. D Street.pdf

Follow Up Flag: Follow up Flag Status: Flagged

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Hello Kristen,

Attached is a picture of our house at 246 D St. I used a bosch laser digital measurement tool to get the most accurate measurement of the house height. I included a picture of the measurement readout in the bottom right corner of the picture. Our house measures 27 Feet and .01". The garage height is 20 Feet and 9 inches. The house is over 6 Feet taller than the garage.

If necessary, we can lower the base of the lot by 2 additional feet. This would put the house at more than 8 Feet taller than the garage. Would only do this if necessary due to the added cost.

We cannot make the building shorter because it needs to accommodate the door height for my box truck to have access.

Making the building any shorter would require a complete redesign and then make it unsuitable for our needs and is cost prohibitive.

The color Grey shown in the picture is what we plan to use on the garage and the roof would be galvalume. This color scheme would neutral and fit in with the overall aesthetics.

Best regards, Ted Elswick



## **Kristen Brown**

From: Fernley Tire & Brake <fernleytire@gmail.com>

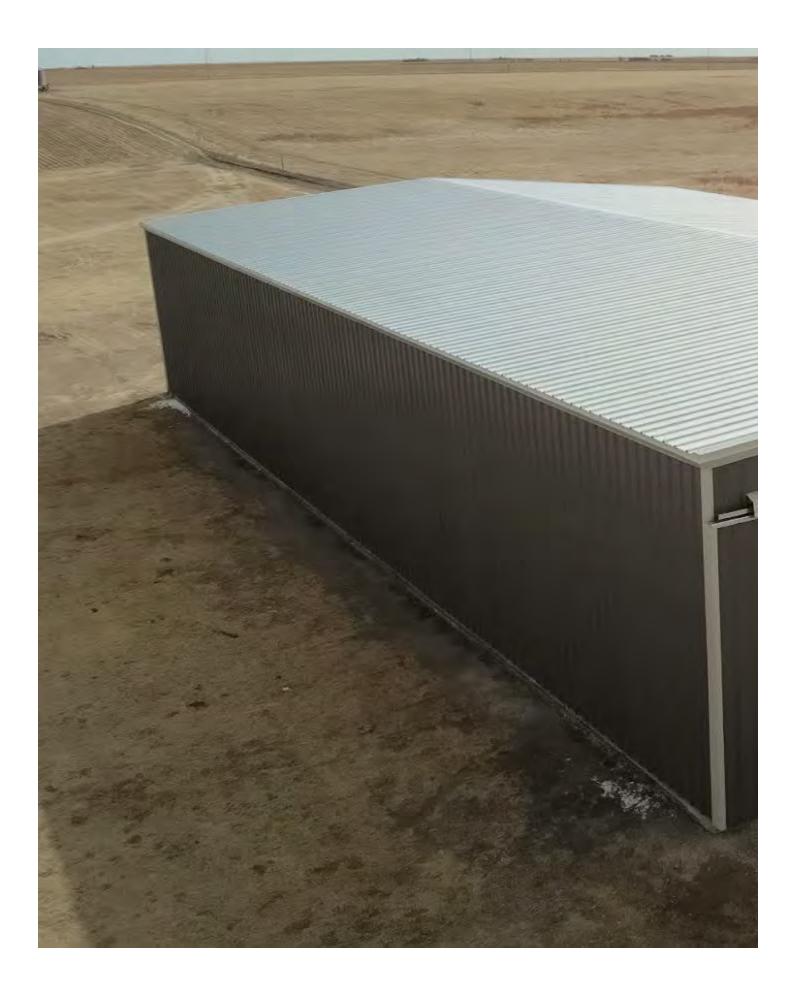
Sent: Wednesday, May 8, 2024 11:28 AM

To:Kristen BrownSubject:Color display

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Here is what the color would look like if we were to use Gray





# ACENDA ITEM O

# COMSTOCK HISTORIC DISTRICT COMMISSION P.O. BOX 128 VIRGINIA CITY, NEVADA 89440

# APPLICATION FOR CERTIFICATE OF APPROPRIATENESS

Pursuant to Nevada Revised Statutes Section 384.110, application is hereby made to the Comstock Historic District Commission for a Certificate of Appropriateness for work I propose to undertake as described below:

propose to undertake as deserved the	
Property address/description_246 D Street_	
Located in the community ofVirgingle	nia City
Description of proposed work:  Mew StructureGarage <u> ろ</u> の' スプ'	
New StructureGarage 30 7 80	
□Alteration of / Addition to Existing Structure	NONE
□Move Existing Structure	
(Reason)	
□Demolish Existing Structure	
(Reason)	
By making this application I hereby agree to indemnify and save and and Storey and their agents and employees from any and all claims application. I further agree to strictly comply with any and all con regulations and laws of the Comstock Historic District Commission	ditions of the Certificate of Appropriateness, if issued, and the
Owner or Designated Representative:	2 29 - 24
Name TED ELSWICK	Date 3-29-24
Mailing Address 1361 HORSE CKE Signature Mis Engl	Telephone 775 835 9785
Signature Ymno CMM	relephone_castor_
CHDC Staff:	Comstock
Received By	Fitle Preservation & Date April 2024 History Officer



Proposed garage location behind house



View from D Street



View from C Street











From: Fernley Tire & Brake
To: Kristen Brown
Subject: Color of Garage

**Date:** Monday, April 22, 2024 8:18:20 PM

**WARNING** - This email originated from outside the State of Nevada. Exercise caution when opening attachments or clicking links, especially from unknown senders.

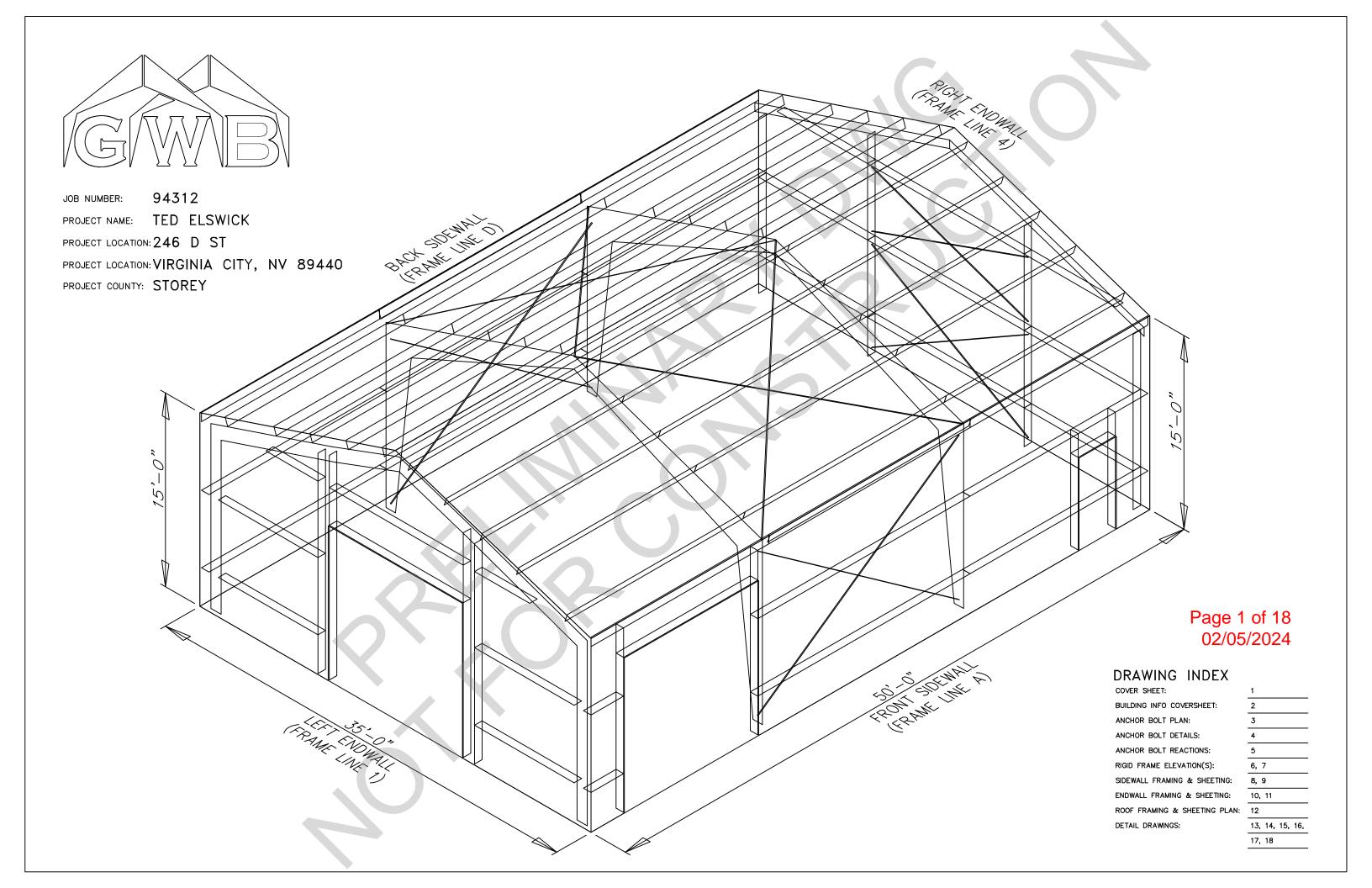
### Hello

The new building for our property at 246 N. D Street Virginia City, NV will be the color of galvalume. This color galvalume will the color of roofing, sides, and will paint roll-up doors silver.

I appreciate your assistance and consideration

Best regards,

Ted Elswick



#### **GENERAL NOTES**

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W. & C) HOT ROLLED STEEL ANGLES (L)	A572	Fy = 50 KSI
		A36	Fy = 36  KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50  KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55  KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HÍGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH

Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & LESS) A307 A325-TYPE 1 HIGH STRENGTH BOLTS (>1"Ø TO 1 1/2"Ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE - 5/8" × 0'-1 3/4" A325-N
B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

#### 1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

Buildings in seismic design category c or lower and/or with crane systems 10 tons or less do not require turn of the nut pre tensioning

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE— BETWEEN ROOF PANELS & RIDGE CAP

- BETWEEN WALL PANELS & EAVE/GABLE TRIM

.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO
PROCURE AND INSTALL AS NEEDED.

#### 1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES

STUCKLAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE MISTIS. THE OFFICE THE OF MINOR MISTIS BY THE USE OF MINITERIST HE OFFI THE OFFI OF MINITERIST. OF THE MINITERIST HE OFFI THE OFF

#### BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE DECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN OF CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEAS FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE LATEST ENTITION)

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. F. COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMI SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED THE ERECTION OPERATION SHALL BE DETERMINED, PURINSHED AND INSTALLED BY THE EINO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STAIL BRACTE LATERS EDITION). PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEA BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO TO

CONCRETE.

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESI
IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATIO
FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT
ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY
REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SULD OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHOR,
FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER
EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING
SYSTEMS MANUAL) SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODER AMOUNTS OF REAMING, CHIPPING, WELDING OR CULTING, AND THE DRAWING OF ELEMENT INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED B' FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHO IS RESPONSIBLE ETHER TO CORRECT THE ERROR OR TO APPROVE THE MOST FEICIENT ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST ENTION).

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OF ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WC SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITIO

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction lead or copper, both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panies. The panels of the pane

2.13 SAFETY COMMITMENT. METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE. 3 SAFETY COMMITMENT. METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVEN PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFE' HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES, DAILY MEET HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFE WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTR TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

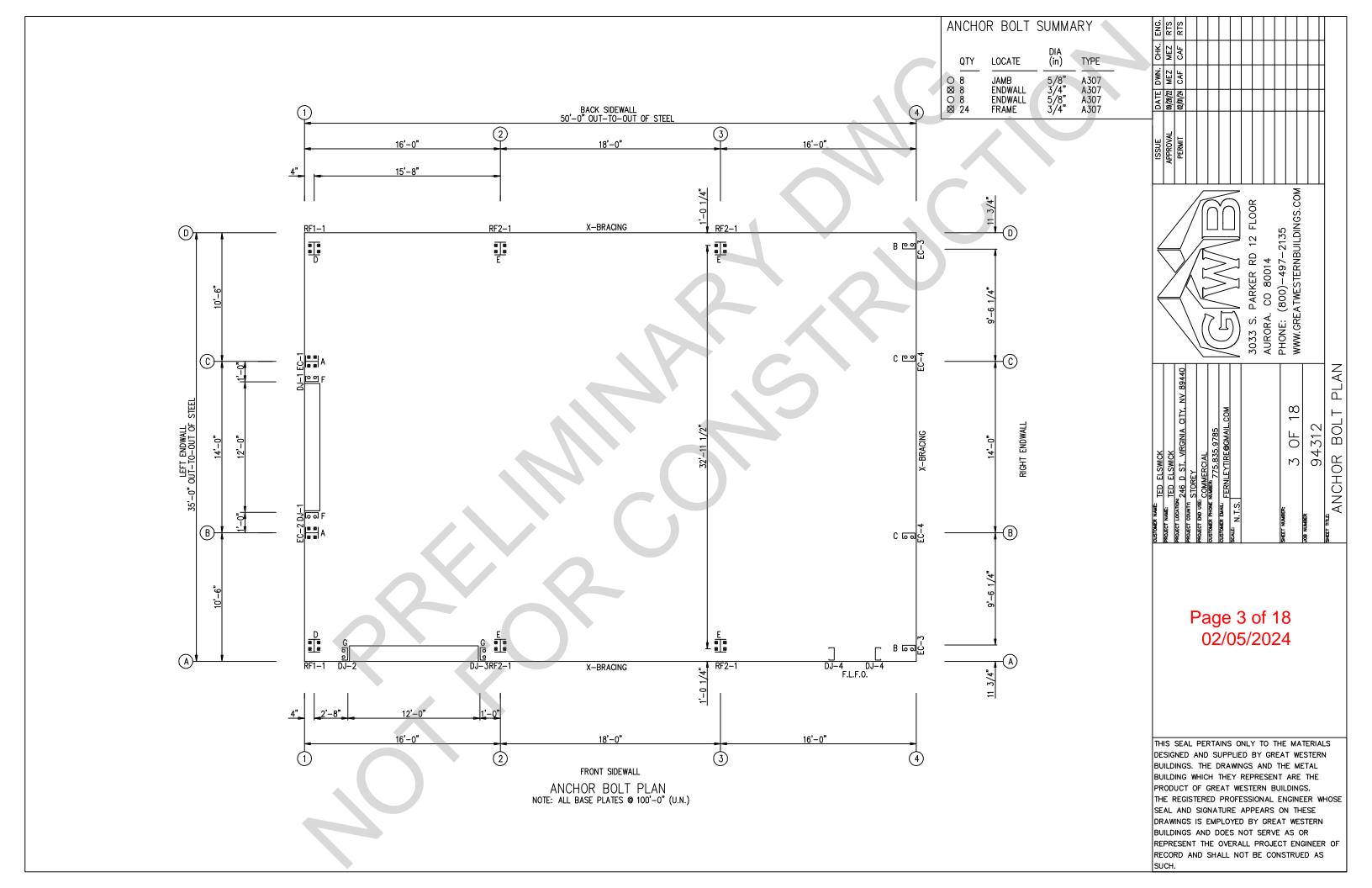
2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DE CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AN ICF BUILDINE. SET TABLE RELOW. ICE BUILDUP, SSE TABLE BELOW.

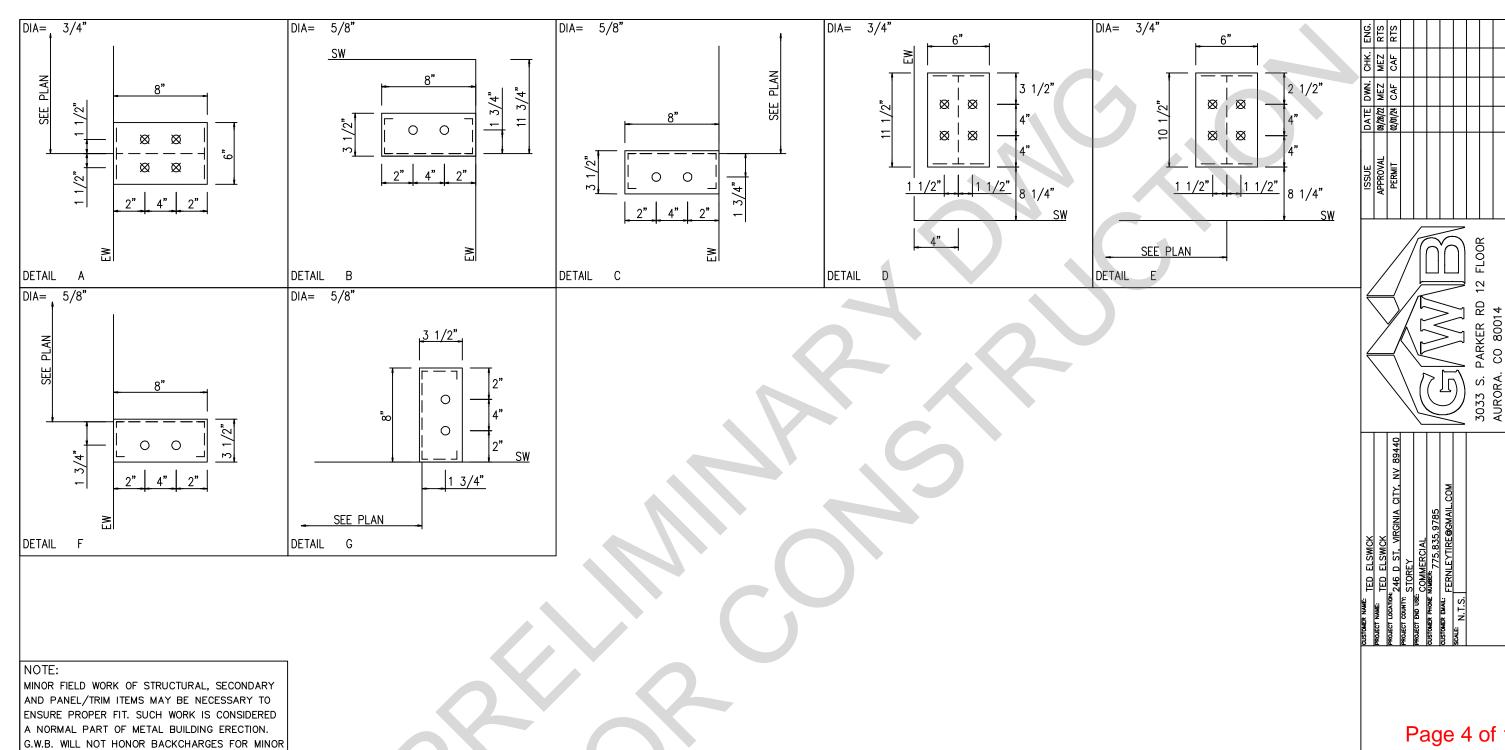
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22,45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 200 EDITION. SECTION A8.4. PAGE XI-A8-2

		BUILDING LOADS	R T S R T S
ION.	THIS STRUCTURE HAS BEEN DESIGNE	D IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:	CHK.
ARE TO BE N CRITERIA EASE FOR TICE,	DESIGN LOADS: DESIGN CODE / WIND CODE OCCUPANCY / RISK CATEGORY	: IBC 18 / IBC 18 : II — Normal	MEZ CAF
S. PLANS ITIES. DING N R A EN OF THE	ENCLOSURE  ROOF DEAD LOAD (D) (PSF)  ROOF COLLATERAL LOAD (C) (PSF)  WIND LOAD  ULTIMATE WIND SPEED, (VULT) (MPH)	: Enclosed : 10.0 : 1.00	APPROVAL 19/28/28 PERMIT 12/01/24
ND TEMPORARY QUIRED FOR E ERECTOR. NG ANCHOR STANDARD	WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF) WIND ENCLOSURE CLASSIFICATION LIVE LOAD	: C : 0.18/-0.18 : 23.90/-25.93 : Enclosed	
TO PERMIT HEAR, 1 BOLT THE DESIGN AND	PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF) SNOW LOAD CROUND SNOW LOAD (Ps.) (PSF)	: 20.00 : No : 20.00	12 FLOOR 2135 UILDINGS.CC
ATION OR HAT BY COLUMN HE SOIL HORAGE AND EER	GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct)	: 70.00 : 70.0 : 1.00 : 1.00 : 1.00	PARKER RD CO 80014 (800)-497- ATWESTERNBI
DDERATE ENTS D BY THE ARE TO BE WHOEVER INT AND ARD	SEISMIC LOAD  SEISMIC IMPORTANCE FACTOR, (Ie) SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY	: 1.00 : d : Ss = 1.586 :S1 = 0.553 : Sds = 1.269 :Sd1 = 0.644 : D	033 S. URORA. HONE: WW.GRE.
OTHERWISE ES, UNLESS WORK IS ETE OR TO TION)	BASIC SEISMIC FORCE RESISTING SYSTEM  TOTAL DESIGN BASE SHEAR, (V) (KIPS)	:STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) :LONGITUDINAL = 19.53	NV 89440
THE ANELS. JLD BE TURE ETY OF M.B.S.	RESPONSE MODIFICATION FACTORS, (R)	: TRANSVERSE = 19.67 : RIGID FRAMES = 3.25 $\Omega$ = 3.00 : SW X-BRACING = 3.25 $\Omega$ = 2.00	SGINIA OF THE STATE OF THE STAT
VENTION AFETY AND ITY, MAKE NG A MEETINGS ITS, RUBBER AFETY NETS	SEISMIC RESPONSE COEFFICIENTS, (Cs)	:RIGID FRAMES = 0.3906 :SW X-BRACING = 0.3906	TED EL TED EL 246 D STORE: STORE: TERNIER 7
ETRUCTION RED OF DEPTH AND/OR	ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	:EQUIVALENT LATERAL FORCE PROCEDURE	OUSTOWER NAME: PROJECT NAME: PROJECT COUNTY: PROJECT COUNTY: PROJECT COUNTY: PROJECT NAME BAME: SCALE. N.T.S. SHEET MIMBER: SHEET MIMBER:
EIGHT START	BUILDING DESCRIPTION:  WIDTH (FT) : 35.0  LENGTH (FT) : 50.0  EAVE HEIGHT AT BSW (FT) : 15.0  ROOF SLOPE AT BSW : 4.0:12  ROOF SLOPE AT FSW : 4.0:12  BAY SPACING (FT) : 1 AT 16, 1 AT 18, 1  COVERING AND TRIMS:  ROOF PANELS & TRIMS  PANEL TYPE : 26 GA. PBR PANEL COLOR : GALVALUME  TRIM COLORS  GABLE/EAVE : ASH GRAY	AT 16	Page 2 of 18 02/05/2024
	EAVE GUTTER : ASH GRAY  WALL PANELS & TRIMS  PANEL TYPE : 26 GA. PBR PANEL COLOR : HAWAIIAN BLUE  TRIM COLORS  CORNER : ASH GRAY FRAMED OPENING : ASH GRAY DOWNSPOUTS : HAWAIIAN BLUE BASE : HAWAIIAN BLUE  INSULATION  ROOF INSULATION : N/A  WALL INSULATION : N/A		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF

REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS





Page 4 of 18 02/05/2024

DETAIL

BOL

ANCHOR

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9431

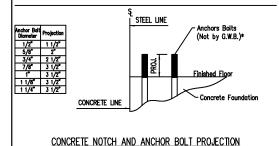
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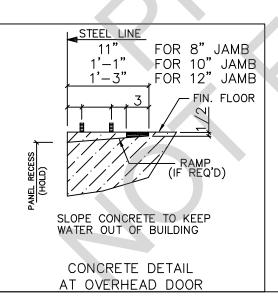
ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

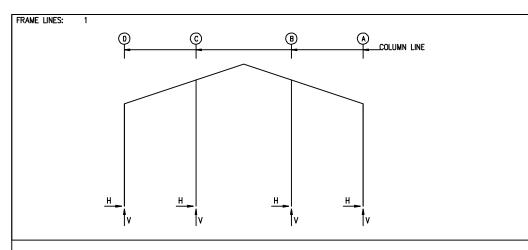
FIELD WORK.

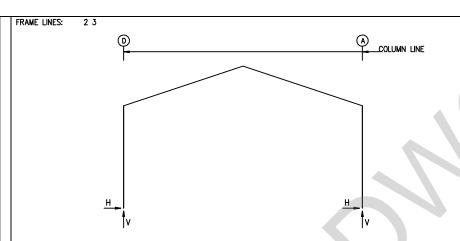
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.









RIGIE	FRAN	<b>⁄</b> Е:	BASI	C COLUM	N REACT	IONS (k )							
FRAME Line 1 1 1	Column Line D A C B	Horz 0.0 0.0 0.0 0.0	Dead Vert 0.7 0.7 1.5 1.5	Horz 0.0	vert Vert 0.0 0.0 0.1 0.1	Horz 0.0	Vert 0.7 0.7 2.2 2.2	Horz 0.1 -0.1 0.0 0.0	-Snow Vert 2.5 2.5 7.5 7.5	Horz -1.1	Left1- Vert -2.0 0.3 -0.2 -2.5	-Wind_ Horz 1.3 1.1 0.0 0.0	Right1- Vert 0.3 -2.0 -2.5 -0.2
FRAME Line 1 1 1	Column Line D A C B	Wind Horz -1.4 -0.9 0.0 0.0	Left2- Vert -1.3 1.0 0.1 -2.2	Horz	Right2- Vert 1.0 -1.3 -2.2 0.1	Wind HorzOP 0.0 0.0 -2.0 -2.0	Vert 0.0 0.0	Wind HorzOP 0.0 0.0 2.2 2.2	0.0 0.0	Wind Horz 0.7 -0.4 0.0 0.0	Long1- Vert -1.0 -1.4 -1.5 -0.2	Wind Horz 0.4 -0.7 0.0 0.0	_Long2- Vert -1.4 -1.0 -0.2 -1.5
FRAME Line 1 1 1	Column Line D A C B	-Seismi Horz -2.2 -2.2 0.0 0.0	ic_Left Vert -4.1 4.1 5.1 -5.1	Seismic Horz 2.2 2.2 0.0 0.0	_Right Vert 4.1 -4.1 -5.1 5.1	-Seism Horz 0.0 0.0 0.1 0.1	Vert 0.0 0.0	F1UNB_ Horz 0.1 -0.1 0.0 0.0	Vert 2.4 0.5	F1UNB_ Horz 0.1 -0.1 0.0 0.0	SL_R- Vert 0.5 2.4 3.4 8.6		
FRAME Line 2* 2*	Column Line D A	Horz	Dead Vert 3.7 3.7	Colla Horz 0.1 -0.1	teral- Vert 0.3 0.3	Horz	Vert	Horz 7.3 -7.3	-Snow Vert 20.8 20.8	Wind Horz -3.5 -1.4	_Left1- Vert -5.9 -3.2	-Wind_ Horz 1.4 3.5	Right1- Vert -3.2 -5.9
FRAME Line 2* 2*	Column Line D A	Horz	_Left2- Vert -3.7 -1.0	−Wind_ Horz 1.0 3.9	Right2- Vert -1.0 -3.7	Wind Horz 0.7 -0.1	_Long1- Vert -6.3 -6.0	Wind Horz 0.1 -0.7	I_Long2- Vert -6.0 -6.3	Horz -4.2	Vert	Seismic Horz 4.2 4.2	_Right Vert 3.5 -3.5
FRAME Line 2* 2*	Column Line D A	-Seismi Horz 0.0 0.0	ic_Long Vert -9.4 -9.4	F2UNB_ Horz 5.8 -5.8	SL_L- Vert 19.8 11.4	F2UNB_ Horz 5.8 -5.8	SL_R- Vert 11.4 19.8						
2*	FRAME lir	ies:	23										

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES														
Frm Line	Col Line	Load Id	Hmax H	imn_Reac V Vmax	tions(k Load Id	) Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)	
1	D	3	1.6	3.7	4	-1.5	-2.6	4	0.750	6.000	11.50	0.375	0.0	
1	Α	5 2	1.5 -1.6	-2.6 3.7	2 5	-1.6 1.5	3.7 -2.6	4	0.750	6.000	11.50	0.375	0.0	
1	С	7 9	1.3 0.0	-0.6 10.2	8 5	-1.2 0.0	-0.4 -3.0	4	0.750	6.000	8.000	0.375	0.0	
1	В	10 9	1.3 0.0	-0.6 10.2	11 4	-1.2 0.0	-0.4 -3.0	4	0.750	6.000	8.000	0.375	0.0	

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES													
Frm Line	Col Line	Load Id	Hmax H	mn_React V Vmax	ions(k Load Id	Hmin H	V Vmin	Bol	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	1	8.6	24.8	4 6	-2.4 0.5	-0.9 -5.0	4	0.750	6.000	10.50	0.375	0.0
2*	A	5 1	2.4 -8.6	-0.9 24.8	1 6	-8.6 -0.5	24.8 -5.0	4	0.750	6.000	10.50	0.375	0.0
2*	FRAME lin	es:	2 3	•									

	INAME	III Co.	2 3												
ENDWA	ALL CC	LUMN:		BASIC C	OLUMN R	EACTION	S (k )								
Line   4   4   4   4		ert 4 3 3	Collat Vert 0.0 0.1 0.1 0.0	Live Vert 0.6 2.2 2.2 0.6	Snow Vert 2.2 7.8 7.8 2.2	_	Wind_Left1 Horz V6 0.0 -0 -1.4 -4 0.0 0.0	ert .6 .2 2		Vert −0.9	Wind_Left2 Horz V 0.0 -0 -1.4 -3 0.0 0. 0.0 -0	ert    .1  .7  .8	Wind_Right2 Horz Ver 0.0 -0.4 0.0 0.8 1.4 -3.7 0.0 -0.1	l −0.7 −1.9 7 −1.9	
Line   4   4   4   4   4   4   4   4   4	Col Su	orz 8 1	0.0 - 0.5 -	Vert -1.2 -1.4 - -1.7	0.0 - -0.5 - 0.0 -	g2 Vert -0.8 -1.7 -1.4 -1.2	Seis_Le Horz 0.0 -4.4 0.0 0.0	ft Vert 0.4 -5.7 5.4 0.0	Seis_I Horz 0.0 0.0 4.4 0.0	Right Vert 0.0 5.4 -5.7 0.4	Seis Long Horz 0.0 0.1 0.1	E2UN Horz 0.0 0.0 0.0 0.0	B_SL_L- Vert 2.2 9.0 3.4 0.4		
Line   4   4   4   4   4	Line Ho A 0. B 0. C 0.	0 3.4 0 9.0	rt  -  -  -	MAXIMUM	REACTIO	NS. ANG	CHOR BOLTS	S. & RA	SE PLATI	FS					
						•		,							
Frm Line	Col Line	Load Id	Hmax H	lumn_Reac V Vmax		Hmin H	V Vmin	Bol QTY	t(in) DIA	Bas Width	se_Plate(in) Length	Thick	Grout (in)		
4	Α	14	0.5	-0.5	8	-0.4	-0.5	2	0.625	3.500	8.000	0.250	0.0		
4	В	1 7 15	0.0 1.3 0.0	2.6 -1.7 10.5	14 11 4	0.5 -1.2 0.0	-0.5 -0.2 -3.5	2	0.625	3.500	8.000	0.250	0.0		
4	С	10 16	1.3 0.0	-1.7 10.5	8 5	-1.2 0.0	-0.2 -3.5	2	0.625	3.500	8.000	0.250	0.0		
4	D	17 1	0.5 0.0	-0.5 2.6	11 17	-0.4 0.5	-0.5 -0.5	2	0.625	3.500	8.000	0.250	0.0		

# NOTES FOR REACTIONS

Build the	ding reactions are based on following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Wind Speed (mph) Wind Speed Exposure Closed/Open Importance Wind Importance Seismic Seismic Soef Seismic Coeff (Fa*Ss)	= = = = = = = = = =	35.0 50.0 15.0,/15.0 4.0:12/4.0:12 10.0 1.00 20.00 70.00 115.00 Enclosed 1.00 D 1.90
ID ——	Description		

	Seismic Coeff (Fa*Ss) = 1.
ID	Description
1	Dead+Collateral+Snow+Slide_Snow
2	1.18Dead+1.18Collateral+0.7Seismic_Left
3	1.18Dead+1.18Collateral+0.7Seismic_Right
4	
5	0.42Dead+0.7Seismic_Right
6	0.42Dead+0.7Seismic_LongL
7	0.6Dead+0.6Wind_Left1+0.6Wind_Suction

8	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
9	Dead+Collateral
10	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
12	Dond J Colleteral J E11IND CL I

12 Dead+Collateral+ETUNB\_SL\_1
3 Dead+Collateral+ETUNB\_SL\_R
14 0.60ead+0.6Wind\_Suction+0.6Wind\_Long1L
15 Dead+Collateral+EZUNB\_SL\_1
16 Dead+Collateral+EZUNB\_SL\_R
17 0.60ead+0.6Wind\_Suction+0.6Wind\_Long2L

ANCHO	OR BOLT	SUMMA	ARY	
QTY	LOCATE	DIA (in)	TYPE	
0 8 8 8	JAMB Endwall	5/8" 3/4"	A307 A307	_
O 8 Ø 24	ENDWALL Frame	5/8" 3/4"	A307 A307	

BUILDING BI	RACING RE	ACTIO	NS			_	
		Reaction	ns(k ) —Seis	mic –	Panel_S (lb/1		
	ine Horz	Vert	Horz	Vert	Wind	Seis	Note
L_EW 1			7				(h)
F_SW A 2 R_EW 4 E	2,3 2.5 3,C 1.4	1.9 1.7	12.7 4.4	9.4 5.3			
B_SW D 3	3,2 2.5	1.9	12.7	9.4			
(h)Rigid frame a	t endwall						
Reactions for sei			_				

Page 5 of 18 02/05/2024

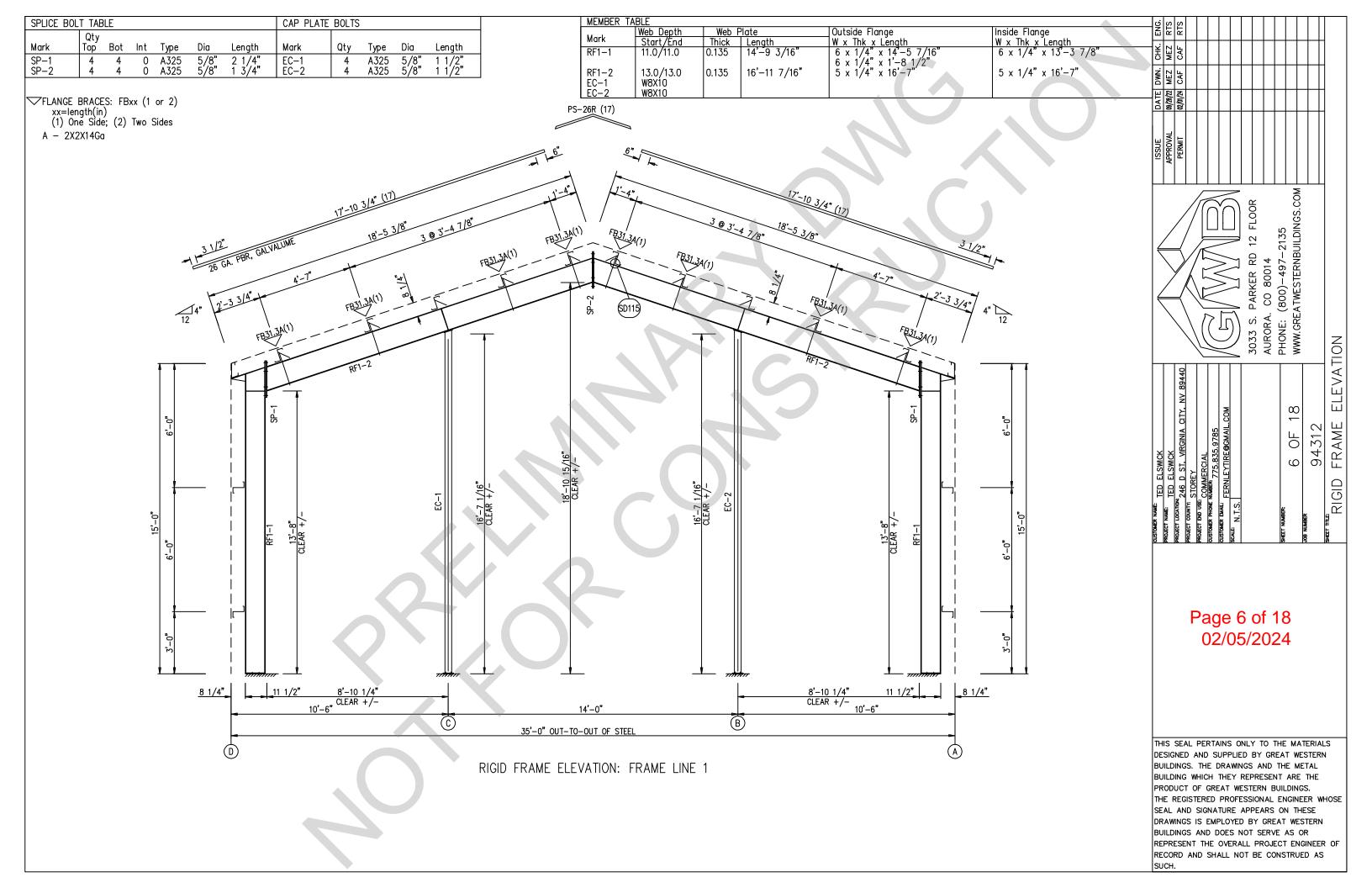
REACTIONS

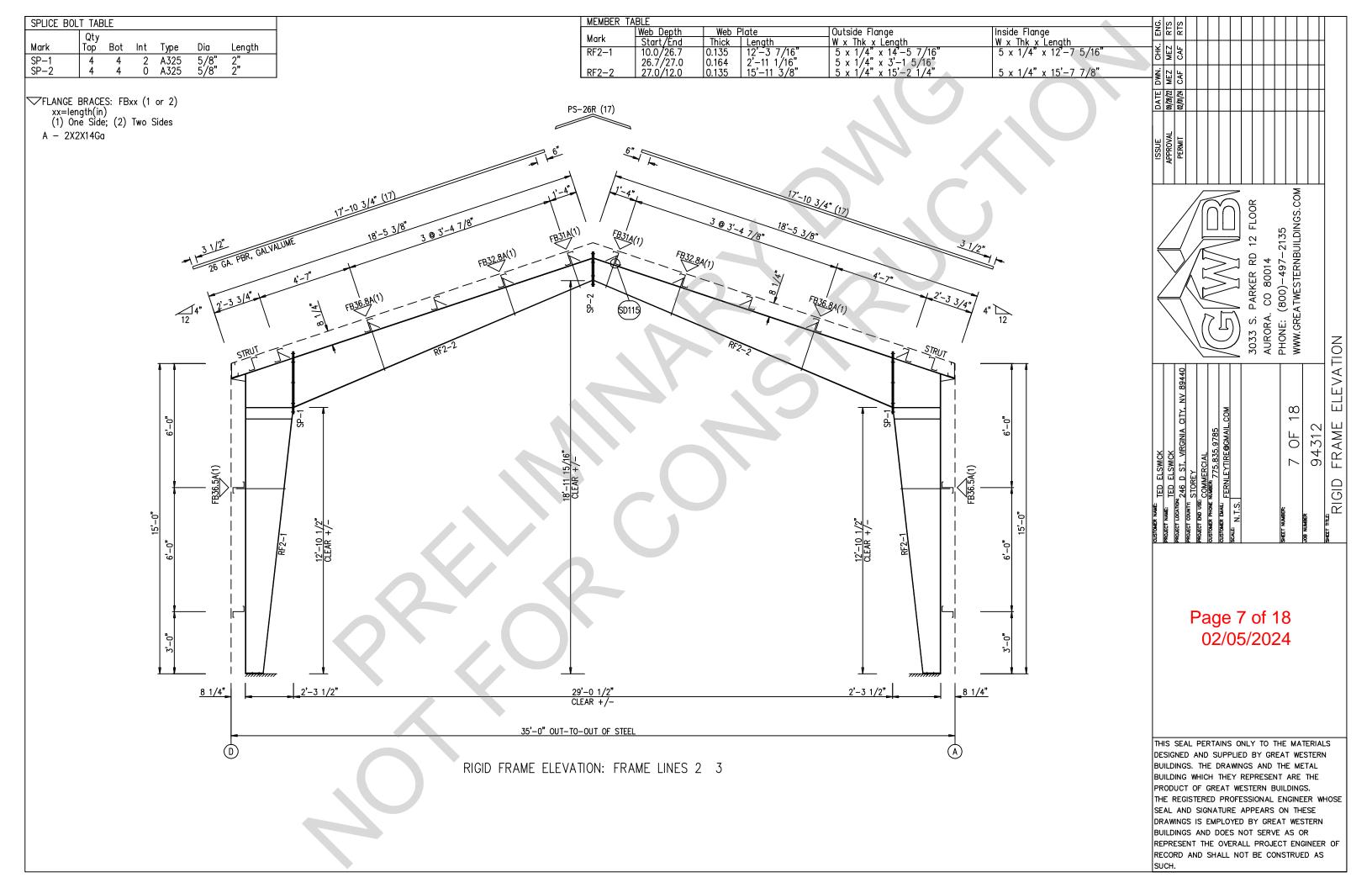
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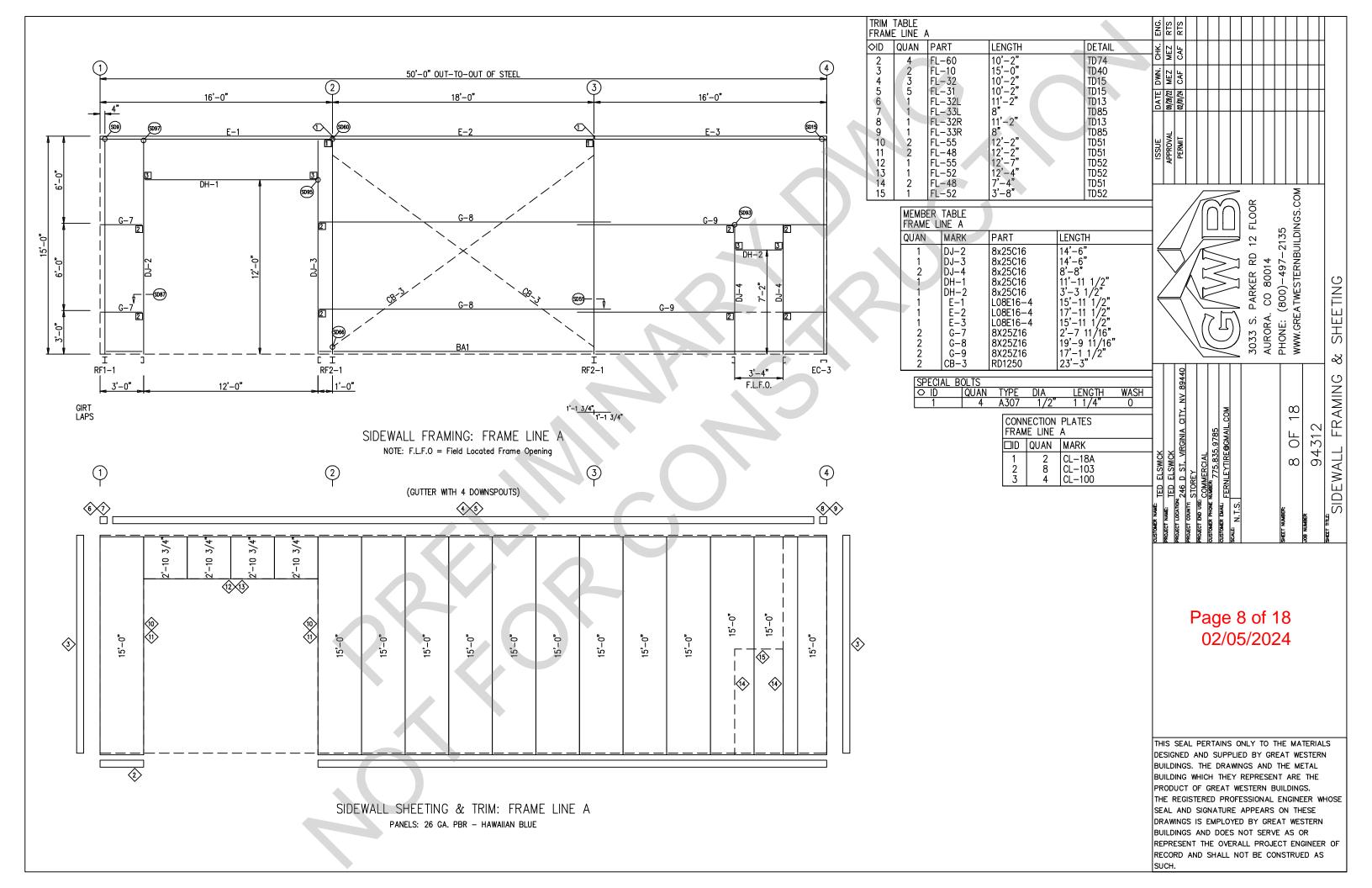
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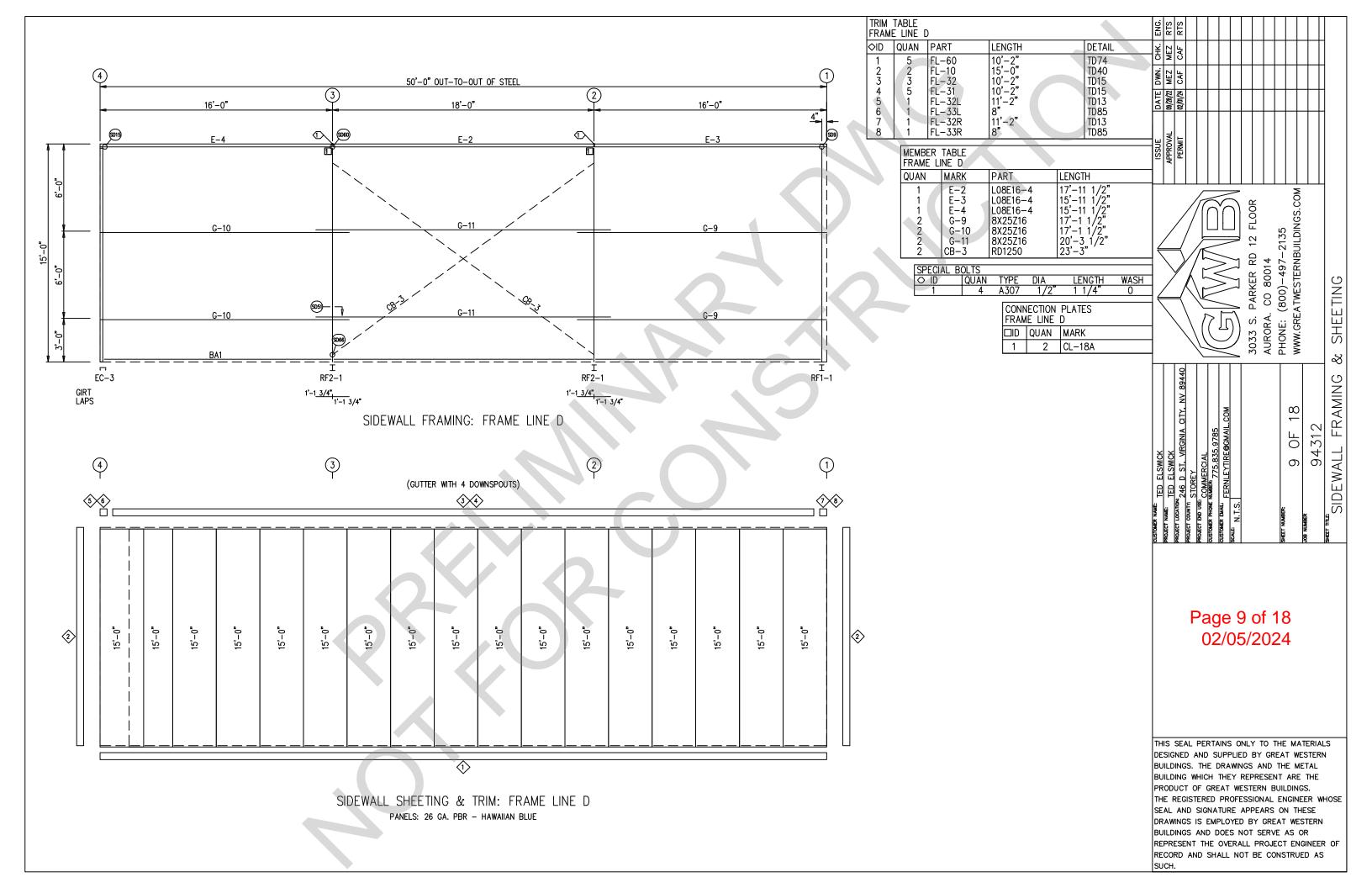
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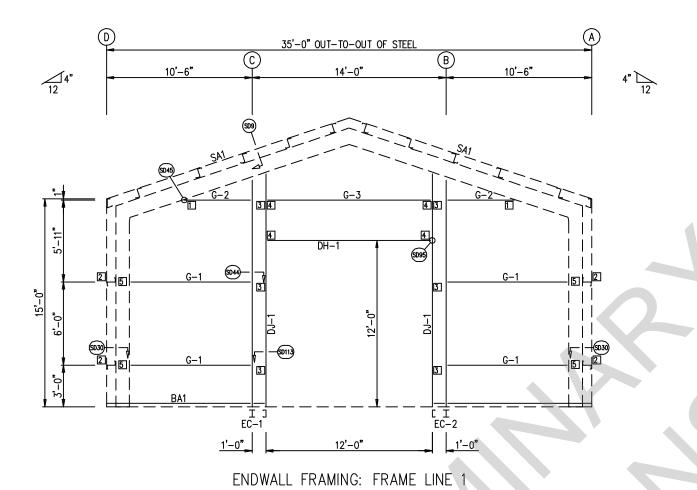
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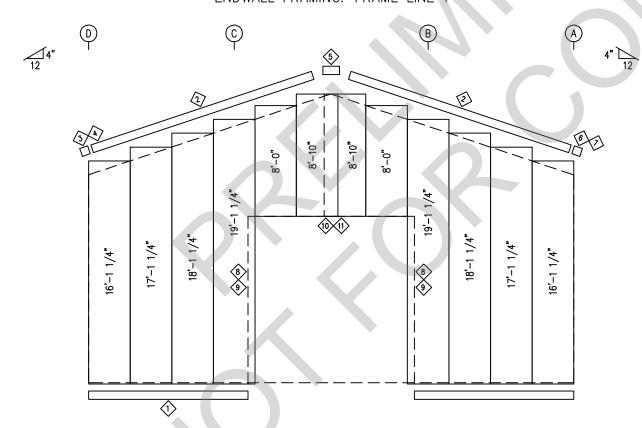






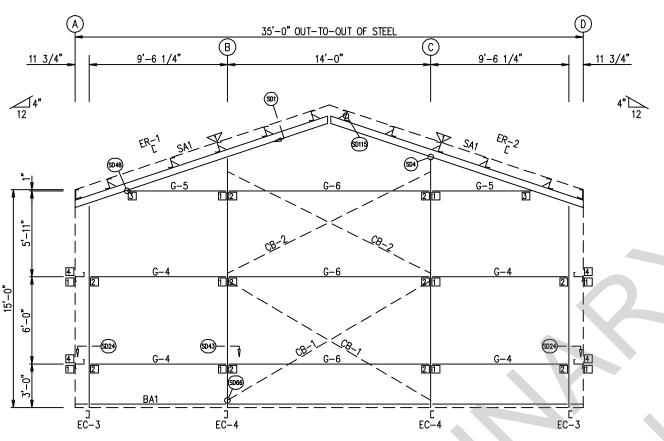




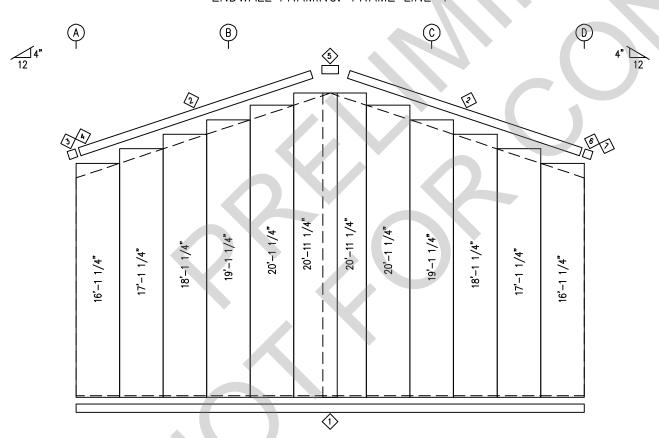


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - HAWAIIAN BLUE

TABLE ME LINE 1						i. Si.	RTS	STS							П	
QUAN	PART	LENGTH		DETA		¥.		CAF							$\parallel \parallel$	
1 Z	FL-60 FL-21 FL-21L FL-328L FL-21R FL-328R FL-55 FL-48	10'-2" 8'-6" 11'-2" 9 1/2" 1'-4" 11'-2" 9 1/2" 12'-2" 12'-2" 12'-7" 12'-4"		TD 74 TD 35 TD 85 TD 13 TD 85 TD 13	5	DATE DWN.	09/28/22 MEZ	02/01/24 CAF								
2 1 1	FL-48 FL-55 FL-52	12'-2" 12'-7" 12'-4"		TD51 TD52 TD52	2	ISSUE	APPROVAL	PERMIT								
BOLT TAE FRAME LI LOCATION COLUMNS	NE 1 /RAFTER	QUAN 4	TYPE A325	DIA 5/8"	LENGTH								F.004 TT	IGS.COM		•
JAMBS/R MEMBE	AFTER  ER TABLE ELINE 1  MARK  EC-1 EC-2 DJ-1 DH-1 G-1 G-2 G-3	PART W8X10 W8X10 8x25C16 8x25C16 8X25Z16 8X25Z16 8X25Z16 8X25Z16	A325	5/8" 5/8" 5/8" ENGTH 6'-7 3/4 6'-10 5/ 1'-11 1/2 '-5 15/1' '-4 7/8" 1'-11 1/2	1 1/2"	<b>€</b>	<u>/</u> _\						PARKER RD 12	PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM		SHEETING
		FRAME	CTION F LINE 1 QUAN N 2 0 4 0 6 0			JUSTOMER NAME: TED ELSWICK		PROJECT LOCATION: 246 D ST, VIRGINIA CITY, NV 89440	ROJECT END USE: COMMERCIAL	JISTOMER PHONE NUMBER: 775.835.9785		N.T.S.		10 OF 18	94312	ENDWALL FRAMING &
						Page 10 of 18 02/05/2024										
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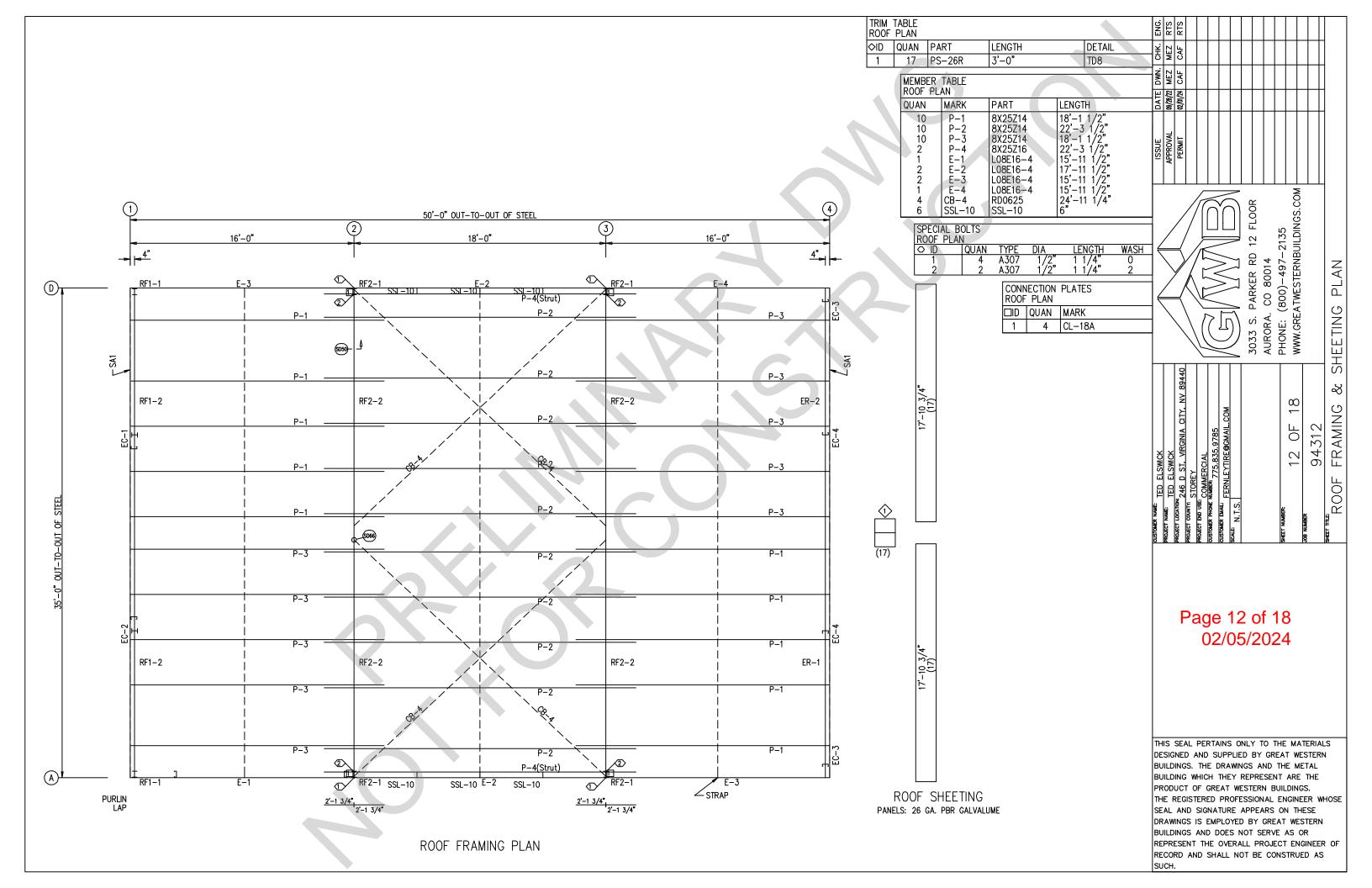
ENDWALL FRAMING: FRAME LINE 4

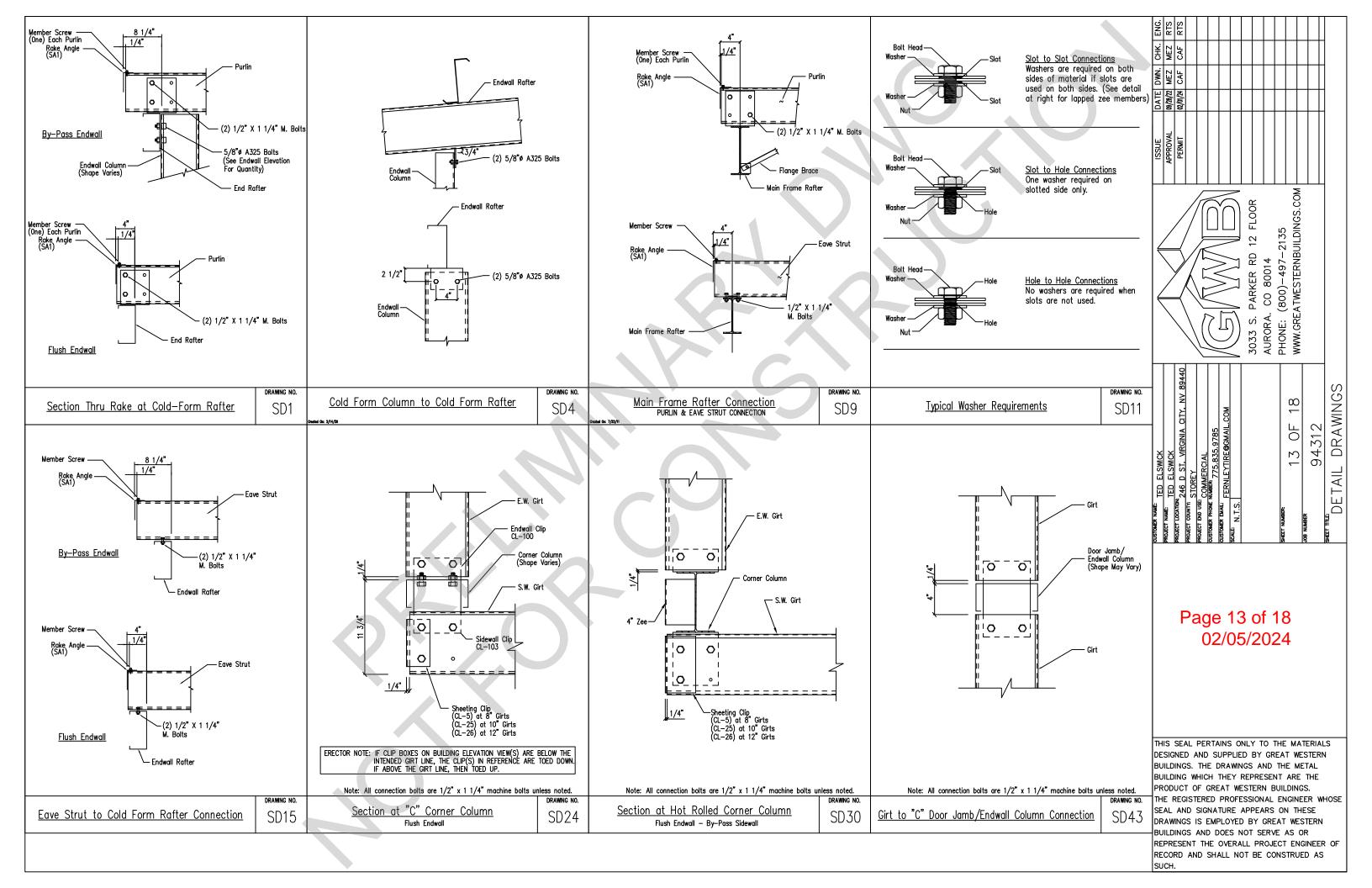


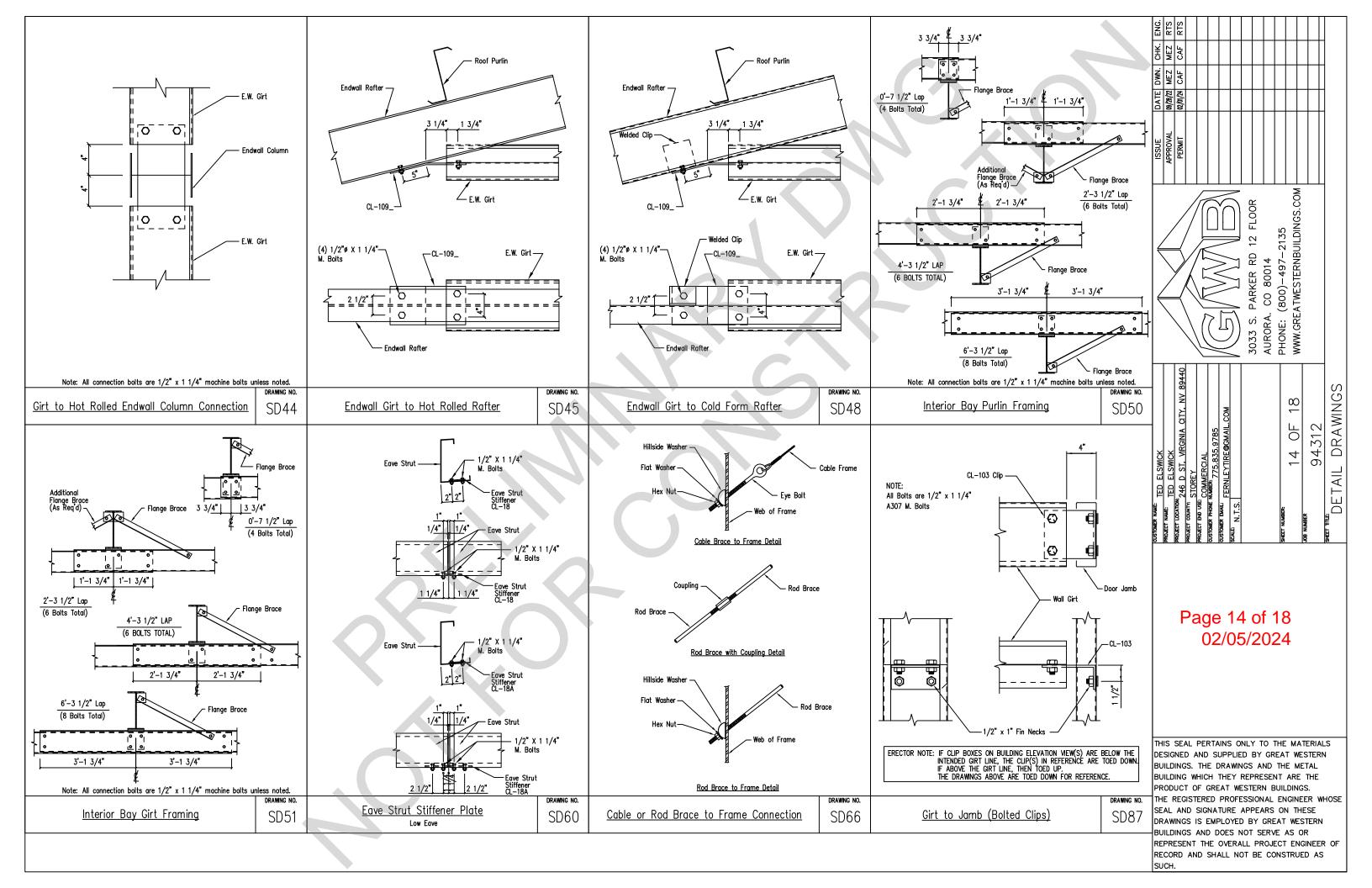
ENDWALL SHEETING & TRIM: FRAME LINE 4

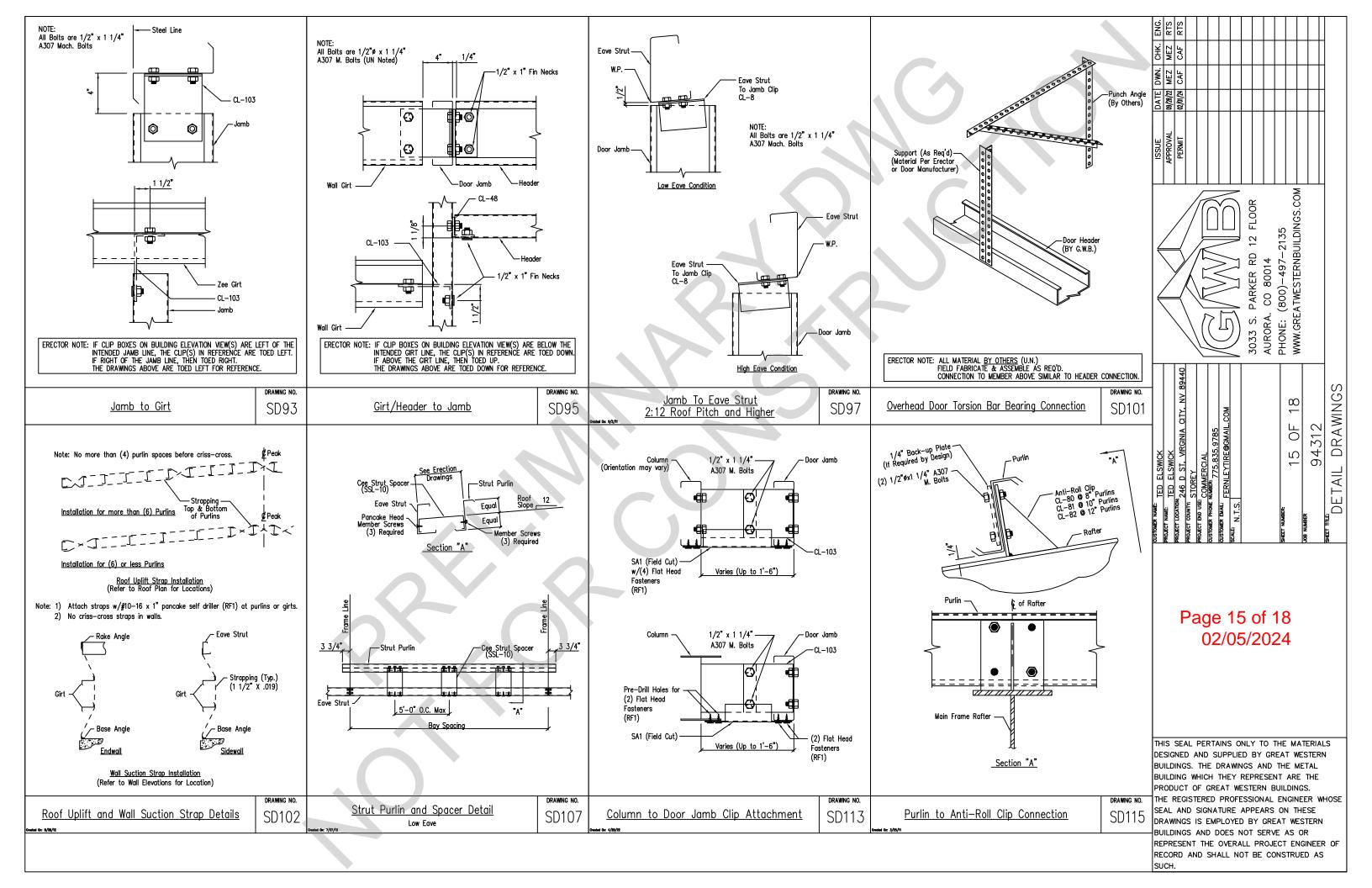
PANELS: 26 GA. PBR - HAWAIIAN BLUE

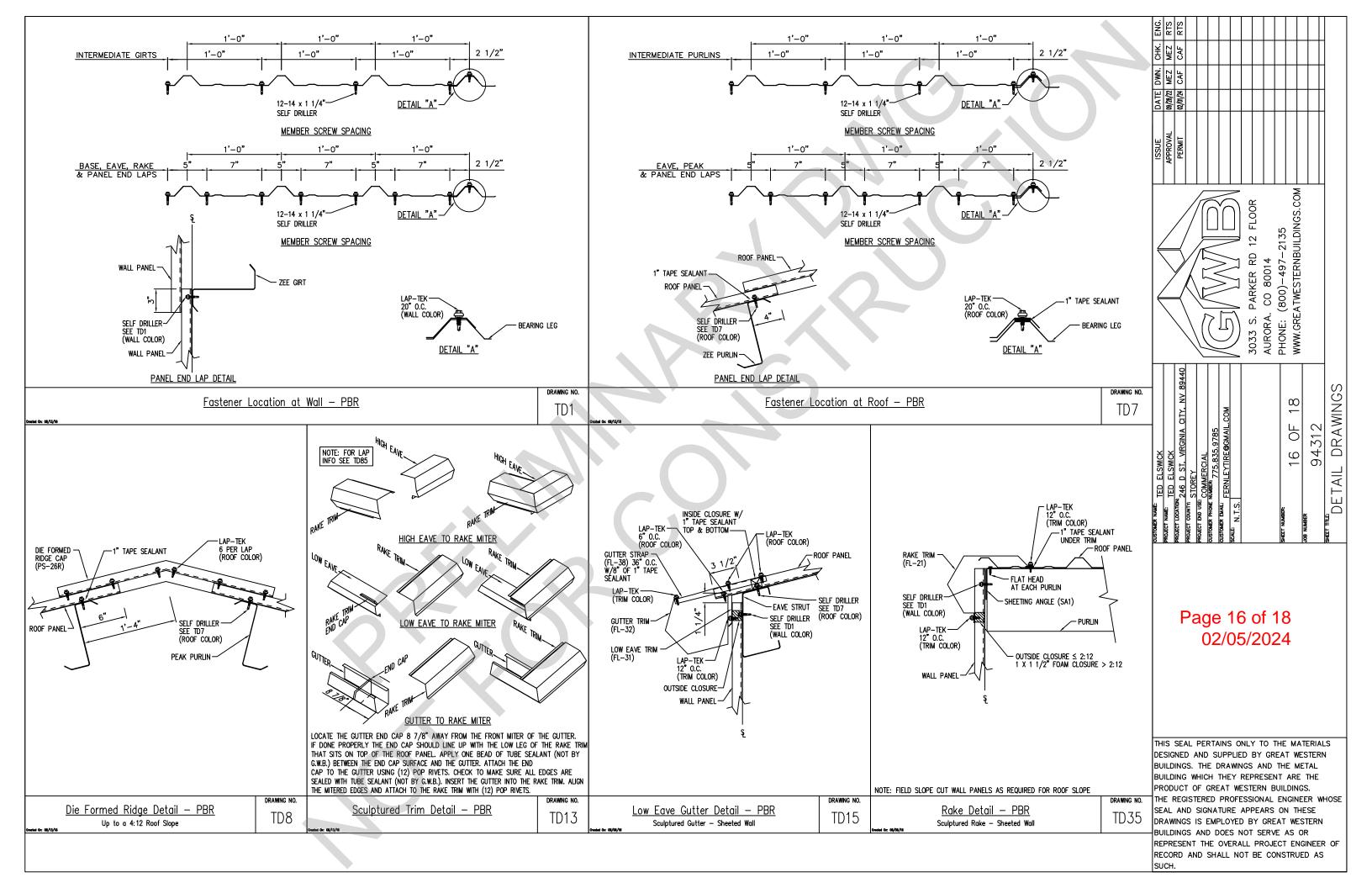
RIM TABLE RAME LINE 4		RNG RT R T R T R T R T R T R T R T R T R T				
ND QUAN PART LENGT	TH DETAIL	CHK.				
1 3 FL-60 10'-2 2 FL-21 8'-6' 3 1 FL-21L 11'-2 4 1 FL-328L 9 1/2 5 1 FL-23 1'-4" 6 1 FL-21R 11'-2 7 1 FL-328R 9 1/2	" TD74 TD35 TD85 TD13 " TD85 TD13	DATE DWN. G  0/8/22 MEZ N  0/8/24 CAF O				
BOLT TABLE FRAME LINE 4  LOCATION  ER-1/ER-2 COLUMNS/RAFTER  MEMBER TABLE FRAME LINE 4  QUAN MARK PART  2 EC-3 8x250 2 EC-4 8x250 1 ER-1 10x25 1 ER-2 10x25 4 G-4 8X250 2 G-5 8X252 3 G-6 8X252 2 CB-1 RD062	AN TYPE DIA LENGTH  A325 5/8" 1 3/4" A325 5/8" 1 1/2"  LENGTH  C16 13'-7 15/16" C12 16'-10" C12 18'-5 1/8" C12 18'-5 1/8" C16 9'-1 15/16" C16 9'-1 15/16" C16 13'-11 1/2" C16 13'-11 1/2" C17 16'-7 1/4"	3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 PHONE: (800)–497–2135 www.GREATWESTERNBUILDINGS.COM	& SHEETING			
1	D QUAN MARK  10 CL-103 2 10 CL-100 3 2 CL-109E 4 4 CL-5  FLANGE BRACE TABLE FRAME LINE 4  VID QUAN MARK  1 2 FB30	1   1	PRAMING			
		Page 11 of 18 02/05/2024				
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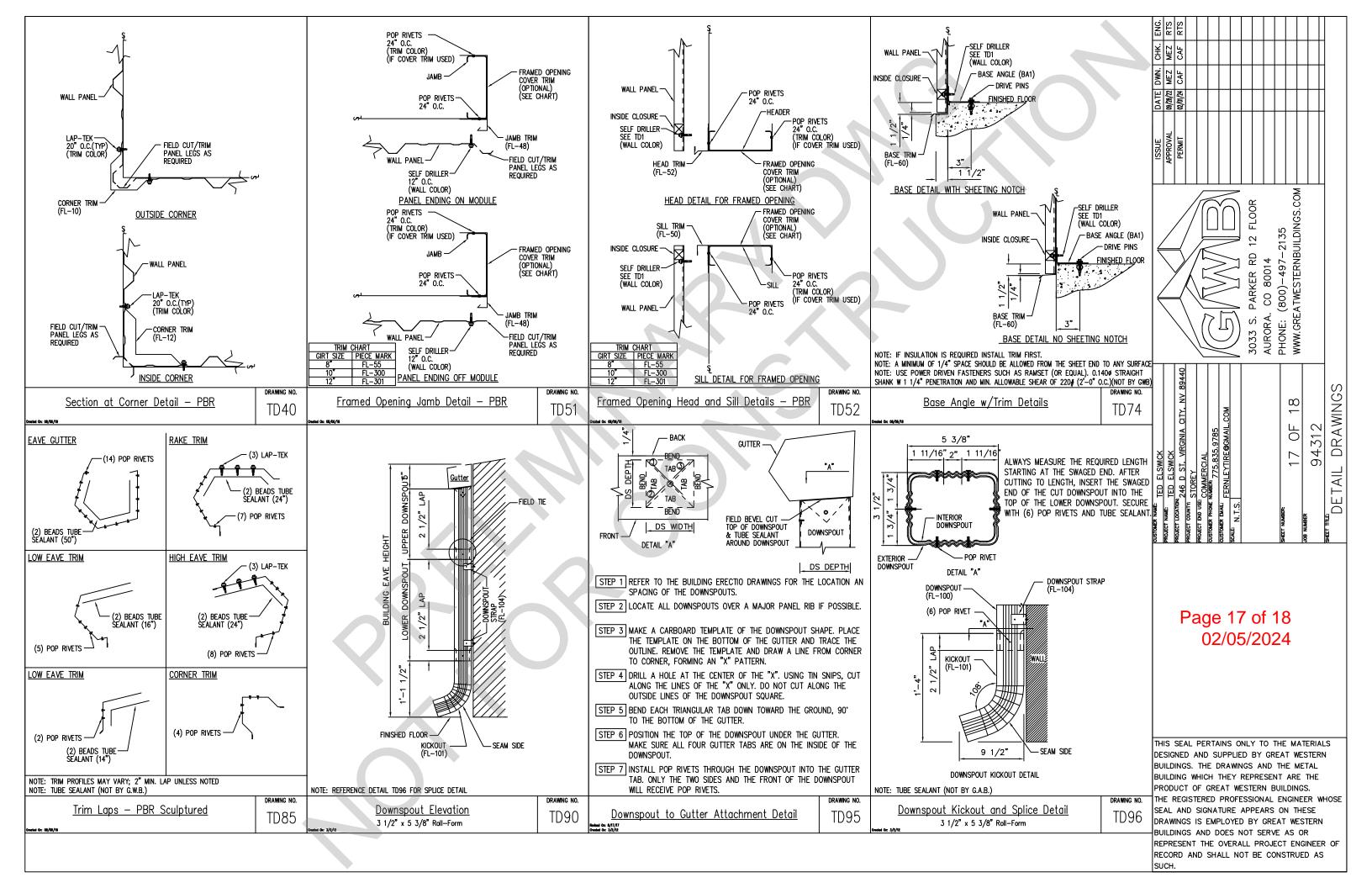


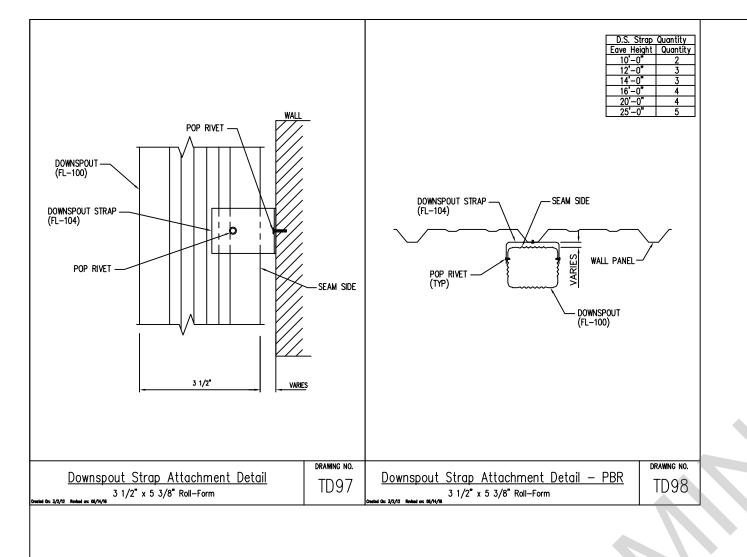


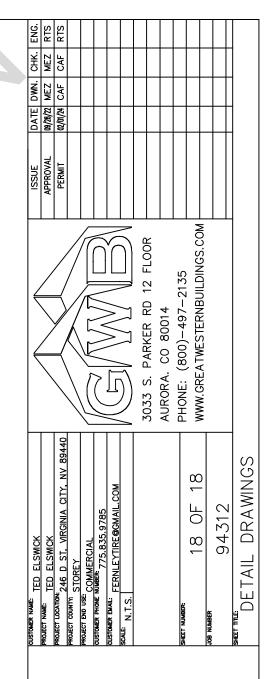












Page 18 of 18 02/05/2024

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